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Contact: Standards and Research Section, Transport Infrastructure Ireland
Postal Address: Parkgate Business Centre, Parkgate Street, Dublin 8, D08 DK10
Telephone: +353 1 646 3600
Email: infoPUBS@tii.ie
Updates to TII Publications resulting in changes to Specification for Road Works Series 500 - Drainage and Service Ducts CC-SPW-00500

Date: September 2017
Page No: 39
Section No: Clause 524 – Pipes for Ducts

Amendment Details:
The following minor amendment has been incorporated into the March 2015 version of this Standard:

a) Clause 524 – Reference to IS EN 50086-2-4 has been deleted as this Standard has been withdrawn.
National Roads Authority

Manual of Contract Documents for Road Works (NRA MCDRW)

AMENDMENT No. 1 (May 2015) to NRA Specification for Road Works Series 500 - Drainage & Service Ducts Dated March 2015

The NRA Specification for Road Works (NRA MCDRW), Series 500 - Drainage & Service Ducts, dated March 2015 is amended as follows:

1. Page 7, Sub-clause 504.4

Delete text ‘tested in accordance with sub-Clause 509.7 and shall be British Standard joints or non-British Standard joints’ from first line and insert text ‘joined in accordance with the manufacturer’s instructions’ in the first line.

2. Page 10, Sub-clause 506.1

Delete text ‘507.18’ and insert text ‘507.14’ in the third line.

3. Page 13, Sub-clause 507.15

Delete text ‘Sub-clause 17’ and insert text ‘Sub-clause 13’ in the fifth line.

4. Page 14, Sub-clause 508.4

Delete text ‘507.7’ and insert text ‘507’ in the second line.

5. Page 14, Sub-clause 508.5

Delete text ‘507.17’ and insert text ‘507.13’ in the third line.

6. Page 14, Sub-clause 508.7

Delete text ‘507.18’ and insert text ‘507.14’ in the second line.

7. Page 14, Sub-clause 508.8

Delete text ‘507.17’ and insert text ‘507.13’ in the fifth line.

8. Page 17, Sub-clause 513.1

Delete the following text;
“1 Permeable backing shall consist of one of the following materials:

(i) A minimum thickness of 300mm of granular material in accordance with Clause 505.

(ii) Geocomposite drains.”

and insert the following text in the first line:

“1 Permeable backing shall consist of one of the following materials:

(i) A minimum thickness of 300 mm of granular material of 20mm nominal size, satisfying
the following criteria:

Piping ratio, defined as

\[
\frac{15 \text{ per cent size of the drainage material}}{85 \text{ per cent size of the backfill material}}, < 5
\]

Permeability ratio, defined as

\[
\frac{15 \text{ per cent size of the drainage material}}{15 \text{ per cent size of the backfill material}}, < 5
\]

where the per cent size of a material is the size of particle corresponding to the given
per cent ordinate of the particle size distribution graph.

(ii) Porous no-fines concrete, cast in situ 225 mm thick complying with the requirements of
the requirements of the 2600 Series.

(iii) Precast hollow concrete blocks complying with IS EN 772-2 & 3 laid in stretcher bond
with dry joints in 225 mm thick walling with holes vertical.”

9. Page 18, Sub-clause 514.6

Delete text ‘4(v)’ from the first line of item (i) and insert text ‘5(v)’.

10. Page 18, Sub-clause 514.6

Delete text ‘Sub-clause 13’ from the third line of item (i), part (b) and insert text ‘Sub-clause 14’.

11. Page 19, Sub-clause 514.7

Delete text ‘5(ii)’ from the second line of the second paragraph and insert text ‘5(v)’.

12. Page 23, Sub-clause 514.16

Delete text ‘14 (vi)’ from the second line of item (a) and insert text ‘14’.

13. Page 23, Sub-clause 514.17

Delete text ’13, 14 and 15’ from the first line and insert text ’14, 15 and 16’.
14. Page 24, Sub-clause 515.3
   Delete text ‘514.4’ from the first line of the first paragraph and insert text ‘514.5’.

15. Page 24, Sub-clause 515.5
   Delete text ‘509.9’ from the third line and insert text ‘509.8’.

16. Page 25, Sub-clause 515.8
   Delete text ‘514.12’ from the third line and insert text ‘514.13’.

17. Page 29, Table 5/10
   Delete text ‘520.11’ from the fifth row of the table and insert text ‘520.1’.

18. Page 30, Table 5/10
   Delete text ‘520.12’ from the third row of the table and insert text ‘520.1’.

19. Page 30, Table 5/10
   Delete text ‘520.13’ from the sixth row of the table and insert text ‘520.1’.

20. Page 35, Sub-clause 519.4
   Delete text ‘radiation in accordance with Sub-clause 517.1’ from the third line.

21. Page 39, Sub-clause 525.8
   Delete text ‘16’ from the second line and insert text ‘14’.

22. Page 40, Sub-clause 525.14
   Delete text ‘15’ from the third line and insert text ‘14’.

23. Page 40, Sub-clause 525.14
   Delete text ‘17’ from the fifth line and insert text ‘14’.
# DRAINAGE AND SERVICE DUCTS

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Drainage and Service Ducts

501 Pipes for Drainage and for Service Ducts

General

1 Where the term drain is used in this Series it shall be deemed to include the terms sewer and piped culvert up to 2000mm internal diameter. Box culverts shall be as described in Appendix 5/1.

2 The Contractor shall design the box or piped culverts listed in Appendix 1/10 in accordance with the requirements of the 100 Series Clause 106 for ‘Structures and Structural Elements and Other Features’ for the Design of Permanent Works by the Contractor.

3 Drains shall comply with this Series and any additional requirements described in Appendix 5/1.

4 Unless otherwise described in Appendix 5/1, only one type of pipe shall be used within any individual drain or service duct between consecutive chambers.

5 The Contractor shall ensure that plastic pipes are not subject to deterioration due to sunlight during the period between manufacture and installation in the ground.

Pipes for Drainage

6 Pipes for drainage shall be selected from the alternatives in Table 5/1 and shall comply with the standards and particular requirements therein. The Contractor shall show that the pipes he selects have hydraulic flow capacity equal to that adopted in the hydraulic design of the system as described in Appendix 5/1. Pipes and fittings other than those included in Table 5/1 shall be permitted provided that they hold a current Irish Agrément Board Roads and Bridges Certificate (or equivalent) stating that they are a suitable alternative for the usage specified in Table 5/1.

As Constructed Information for Pipes Drainage

7 On completion of the whole of the drainage works, the Contractor shall provide the Employer’s Representative with a schedule showing details of all pipe types used, including quality, joints and name of manufacturer.

Table 5/1: Pipes for Drainage

<table>
<thead>
<tr>
<th>Material</th>
<th>Usage</th>
<th>Standard</th>
<th>Particular Requirements</th>
</tr>
</thead>
<tbody>
<tr>
<td>Vitrified Clay</td>
<td>Foul Drains</td>
<td>BS 65 or IS EN 295</td>
<td>“Normal” pipes as defined in BS 65</td>
</tr>
<tr>
<td>Concrete</td>
<td>Foul and surface water drains</td>
<td>IS 6 or BS 5911-1 or IS EN 1916</td>
<td></td>
</tr>
<tr>
<td></td>
<td>not exceeding 900mm internal</td>
<td></td>
<td></td>
</tr>
<tr>
<td></td>
<td>diameter</td>
<td></td>
<td></td>
</tr>
<tr>
<td></td>
<td>Surface water drains not</td>
<td>IS 6 or IS 166 or BS 5911-110</td>
<td>For use with joints complying with Sub-clause 504.4</td>
</tr>
<tr>
<td></td>
<td>exceeding 900mm diameter</td>
<td></td>
<td></td>
</tr>
<tr>
<td></td>
<td>Filter Drains</td>
<td>IS 6 or IS 166 or BS 5911-1</td>
<td>Unperforated not exceeding 2m in length with open joints or castellated rebated joints with the total slot area between castellations being at least 100mm(^2) per metre length of pipe OR Perforated with circular holes not</td>
</tr>
<tr>
<td>Material</td>
<td>Usage</td>
<td>Standard</td>
<td>Particular Requirements</td>
</tr>
<tr>
<td>----------------------------------</td>
<td>------------------------------------</td>
<td>----------------------------------------------</td>
<td>------------------------------------------------------------------------------------------</td>
</tr>
<tr>
<td>Drains exceeding 900mm diameter</td>
<td>IS 6 or IS 166 or BS 5911-1 and IS EN 1916</td>
<td>Pipes having a concrete mix meeting the following requirements (Testing in accordance with the relevant BS and sampling in accordance with Clause 509): A total chloride ion content as described in Appendix 1/10 AND A 28-day minimum works cube strength of not less than 50 N/mm²</td>
<td></td>
</tr>
<tr>
<td>Iron</td>
<td>Foul and surface water drains</td>
<td>BS 437 (Cast Iron)</td>
<td></td>
</tr>
<tr>
<td></td>
<td></td>
<td>IS EN 598 (Ductile Iron)</td>
<td></td>
</tr>
<tr>
<td>Unplasticised polyvinyl-chloride (UPVC)</td>
<td>Foul and surface water drains</td>
<td>IS EN 1401 or BS 4660 or BS 5481 or IS EN 13476 or IS EN 13598-1</td>
<td></td>
</tr>
<tr>
<td></td>
<td>Filter Drains</td>
<td>IS EN 1401 or BS 5481</td>
<td>Perforated with not less than 100mm² of holes per metre length of pipe. The perforations shall not reduce the pipe stiffness by more than 5%. Circular perforations not greater than 10mm or less than 3mm in diameter or rectangular slots not greater than 4mm or less than 0.6mm in width.</td>
</tr>
<tr>
<td>Plastics</td>
<td>Surface water drains</td>
<td>IS EN 13476 or Irish Agrément Board Roads and Bridges Certificate (or equivalent)</td>
<td>Unperforated with watertight joints and with a pipe stiffness and impact resistance as described in Appendix 5/1</td>
</tr>
<tr>
<td></td>
<td>Filter drains</td>
<td>Irish Agrément Board Roads and Bridges Certificate (or equivalent)</td>
<td>Perforated and with a pipe stiffness and impact resistance as described in Appendix 5/1.</td>
</tr>
<tr>
<td></td>
<td>Subsoil field drains</td>
<td>BS 4962</td>
<td></td>
</tr>
</tbody>
</table>
502 Excavation for Pipes and Chambers

1 Excavation shall comply with the General Requirements of the 600 Series and with the following:

(i) soft spots existing below the bottom of an excavation shall be removed and the resulting
voids backfilled and compacted with Granular Material Type B complying with Clauses 803
or 804 or pipe bedding material complying with Clause 503, or mix ST1 concrete;

(ii) any additional excavation below the bottom of an excavation that is required because the
Contractor has allowed the bottom of the pipe trench to become soft or otherwise
unacceptable for the construction of the pipeline or chambers shall be made good as
described in Sub-clause 1(i) of this Clause;

(iii) any excavation greater than the net volume required for the permanent Works below the
level of any pipe surround shall be made good as described in (i) above or Series 600 Class
1, Class 2, or Class 6F material where minimum layer thickness and compaction
requirements can be complied with.

(iv) pipelines and chambers shall not be executed and completed in or beneath areas of
embankment or fill until the execution and completion of the embankment has reached a
level so as to ensure that there is sufficient cover to the top of the pipeline to cater for loading
for residual works, in accordance with NRA HD 140 Determination of Pipe and Bedding
Combinations for Drainage Works..

503 Bedding, Laying and Surrounding of Pipes

1 Immediately following the excavation of the trench, the pipes shall be laid and jointed on the pipe
bed. Pipes shall be laid so that each one is in contact with the bed throughout the length of its
barrel. The pipes shall be laid at the level and gradients shown on the drawings and schedules in
the contract. The deviation in level from that specified at any point shall not exceed 20mm and
in addition the algebraic difference of the deviation in level at any two points on each pipe shall
not exceed 30mm. In the case of socketed or sleeve jointed pipes the bed shall be cut away and
removed at each socket or sleeve to give a clearance of at least 50mm, or 100mm for trenches in
material designated as Hard Material, so that the socket or sleeve does not bear on the bed. Pipes
shall be laid on setting blocks only where a concrete bed or cradle is used.

2 Pipes and fittings shall be examined for damage and the joint surfaces and components shall be
cleaned immediately before laying. Measures shall be taken to prevent soil or other material from
entering pipes, and to anchor each pipe to prevent movement before the work is complete.

3 Pipes complying with BS 4962, which are corrugated coilable perforated pipes, shall be laid only
by automatic single pass drain laying machines where practicable.

4 Drainage pipe and bedding combinations shall be selected from the alternatives described in
Appendix 5/1. Pipe bedding, haunching and surrounding material shall be constructed as shown
on the NRA Road Construction Details listed in Appendix 5/7, and shall comply with the
following:

(i) For pipes on bed material shown on the NRA Road Construction Details listed in Appendix
5/7 the material shall comply with the following:

(a) natural granular material, natural granular material incorporating recycled
aggregate or light weight aggregates shall have a grading in accordance with Table
5/3;
(b) natural granular material, natural granular material incorporating recycled aggregate or light weight aggregates shall have a water-soluble sulphate content of less than 0.38% of sulphate (as SO₃) when tested in accordance with IS EN 1744-1, Clause 10;

(c) concrete shall be mix ST4, backfilling shall not be carried out until after a visual assessment has been undertaken by the designer in order to establish if the concrete has cured sufficiently to receive backfill material.

(ii) For pipes on beds specified in Appendix 5/1 the granular material shall comply either with Sub-clause 3(i) of this Clause or with Table 5/4, with the water soluble sulphate content complying with (i) above.

(iii) Except for filter drains any Class 1 or Class 2 material above the bed, haunch and surround described above, shall be provided to a height of 300 mm above the top of the surround in compliance with the 600 Series, with the maximum particle size of the fill material no more than two thirds of the compacted layer thickness except that lumps of clay and cobbles having an equivalent diameter or more than 150mm shall not be deposited.

(iv) Unless otherwise described in Appendix 5/1 the materials used for the bedding, haunching and surrounding of filter drains shall comply with the appropriate bedding, haunching and surrounding materials specified in Sub-clauses 503.3.(i) to 503.3.(iv) and with the requirements for backfilling specified in Sub-clause 505.3.

Table 5/3: IS EN 13242, Coarse aggregate for pipe bedding, haunching and surrounding material

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<thead>
<tr>
<th>IS EN 13242, Coarse aggregate</th>
<th></th>
</tr>
</thead>
<tbody>
<tr>
<td>Category for general grading requirements</td>
<td>G. 80-20</td>
</tr>
<tr>
<td>Category for tolerances at mid-size sieves</td>
<td>GTNR (no requirements)</td>
</tr>
<tr>
<td>Category for maximum values of fines content</td>
<td>Gravel- f₁₂, Crushed rock, recycled Aggregate – f₄</td>
</tr>
<tr>
<td>LA Abrasion (LA)</td>
<td>LA₅₀</td>
</tr>
<tr>
<td>Water Soluble Sulfate (SS) as SO₃ %</td>
<td>SS₀₃₈</td>
</tr>
<tr>
<td>Nominal pipe diameter, mm</td>
<td>Aggregate size, mm</td>
</tr>
<tr>
<td>Graded</td>
<td>Single sized</td>
</tr>
<tr>
<td>Not exceeding 140</td>
<td>4/10</td>
</tr>
<tr>
<td>Exceeding 140 but not exceeding 400</td>
<td>2/14 or 4/20, 4/10, 6/14 or 10/20</td>
</tr>
<tr>
<td>Exceeding 400</td>
<td>2/10, 4/20, 4/10, 6/14, 10/20 or 20/40</td>
</tr>
</tbody>
</table>
Table 5/4: IS EN 13242, Fine and all-in aggregate for pipe bedding, haunching and surrounding material

<table>
<thead>
<tr>
<th>IS EN 13242, Fine and all-in aggregate</th>
<th>Fine</th>
<th>All-in</th>
</tr>
</thead>
<tbody>
<tr>
<td>Category for general grading requirements</td>
<td>Category $G_{80}$</td>
<td>Category $G_{80}$</td>
</tr>
<tr>
<td>Category for tolerances on manufacturer’s declared typical grading</td>
<td>$GT_{1}NR$ (no requirement)</td>
<td>$GT_{1}NR$ (no requirement)</td>
</tr>
<tr>
<td>Category for maximum values of fines content</td>
<td>Gravel – $f_3$</td>
<td>Crushed rock – $f_{11}$</td>
</tr>
<tr>
<td>LA Abrasion (LA)</td>
<td>LA$_{20}$</td>
<td></td>
</tr>
<tr>
<td>Water Soluble Sulfate (SS) as SO$_3$ %</td>
<td>SS$_{0.8}$</td>
<td></td>
</tr>
<tr>
<td>Nominal pipe diameter, mm</td>
<td>Aggregate size, mm</td>
<td></td>
</tr>
<tr>
<td>Not exceeding 140</td>
<td>0/1, 0/2, 0/4</td>
<td>0/10</td>
</tr>
<tr>
<td>Exceeding 140 but not exceeding 400</td>
<td>Or 0/6</td>
<td>0/10 or 0/20</td>
</tr>
<tr>
<td>Exceeding 400</td>
<td>0/10, 0/20 or 0/40</td>
<td></td>
</tr>
</tbody>
</table>

(v) Granular materials used for bedding, haunching and surrounding of pipes shall have a resistance to fragmentation in Category LA$_{50}$ in accordance with IS EN 13242.

5 Except where the pipeline is to be tested in compliance with Clause 509 before backfilling, the completion of the bedding, haunching and surrounding of the pipes is to be carried out immediately after jointing. The bed, haunch and surround shall be brought up equally on both sides of the pipe ensuring that it is in contact with the underside of the pipe barrel. Granular bedding material is based on being more or less self-compacting and therefore shall be placed in layers not exceeding 150mm thickness and visually assessed in order to confirm adequate compaction of the material and particularly compaction next to the trench walls has been achieved. Pipes shall be maintained to line and level during the bedding, haunching and surrounding operations. Where pipelines are to be tested before being covered, the bedding, haunching and surrounding material shall only be brought up sufficiently to support the pipeline and the joints shall be left exposed until the test is completed satisfactorily.

504 Jointing of Pipes

1 Rigid joints shall mean joints made solid by caulking the sockets, or bolting together flanges integral with the pipes. Flexible joints shall mean joints made with deformable rings or gaskets held between pipe spigots and sockets, sleeves or collars.

2 Joints in surface water drains shall be watertight complying with Sub-clause 3 of this Clause or partly watertight complying with Sub-clause 4 of this Clause as described in Appendix 5/1. Foul drains shall have watertight joints. Filter drains shall have joints complying with Sub-clause 6 of this Clause.

3 Watertight joints shall comply with the appropriate European Standards, the manufacturer’s instructions and the following:

(i) Rigid joints shall be used only where permitted in Appendix 5/1. Spigots and sockets of rigid joints may be caulked with tarred rope yarn or equivalent and the socket completely filled with mortar designation (i) complying with Clause 2404, excluding lime; a fillet of mortar being worked around the socket extending for a length of not less than 50mm from the face
of the socket. Iron pipes with open sockets shall have rigid joints caulked with lead wool or equivalent.

(ii) Joints in UPVC pipes shall not be made with plastic solvent.

(iii) Flexible mechanical joints may be used with surface water pipes complying with BS 65 for vitrified clay pipes.

(iv) Joints for cast iron pipes to BS 437 shall comply with IS EN 877.

(v) Joints in plastics pipes which have an Irish Agrément Board Roads and Bridges Certificate or British Board of Agrément Roads Certificate for Highway Drainage Products shall comply with the manufacturer’s instructions.

4 Partly watertight joints for surface water drains shall be joined in accordance with the manufacturer’s instructions. Push fit joints shall have a register to ensure that the pipe is fully pushed into the joint.

5 Where a concrete bed, cradle, arch or surround is used with rigid pipes having flexible joints, joint filler board complying with Clause 1015 shall be placed in contact with the end of the socket at a pipe joint and shall extend through the full thickness of the concrete in contact with the pipe. Such joints in the concrete bed, haunch or surround shall be at intervals not exceeding 5 metres except where the spacing of joints in the pipe exceeds 5 metres when they shall be at each pipe joint.

6 Joints in pipes for filter drains shall comply with the appropriate European Standard and with the following:

(i) Non-porous and unperforated concrete pipes with spigot and socket, rebated or ogee joints shall be laid with unsealed joints and with a gap of 10mm between the end of the pipe and the inner end of the socket or rebate. The pipes shall be supported with tarred rope yarn or equivalent flexible jointing material within the sockets over the lower third of the circumference so that there are no vertical steps greater than construction tolerances permitted under clause between one pipe and another. Such pipes shall only be used with Type B filter material as described in Clause 505.

(ii) The ends of perforated or castellated concrete pipes with rebated joints shall be pushed tightly together. The width of slots measured along the length of the pipeline formed by jointing castellated pipes shall not exceed 10mm.

(iii) Perforated or slotted UPVC pipes with spigots and sockets or sleeves may be dry-jointed or jointed as described in Sub-clauses 3 and 4 of this Clause.

(iv) Other perforated pipes shall be jointed as unperforated pipes of the same material.

1 Amended as per Amendment No. 1, item 1.
505 Backfilling of Trenches and Filter Drains

1 Backfilling shall be undertaken immediately after the specified operations preceding it have been completed. The material as described in the Contract shall be deposited in layers, compacted in accordance with requirements specified in Clause 601.8 of the 600 Series. Care should be taken to compact the material evenly without dislodging or damaging pipes. Power rammers are not to be used within 300mm of any part of the pipe or joint.

2 Except where otherwise described in Appendix 5/1, trenches other than filter drain trenches shall be backfilled above the pipe surround material described in Clause 503 with Class 1 or 2 general fill material complying with the requirements of the 600 Series. Backfill of trenches, other than in carriageways and other paved areas, shall be brought up to ground level. Where the ground surface on the line of the trench consists of topsoil, the upper section of the backfill shall be topsoil of the thickness described, or of the same thickness and quality of soil as the surrounding ground. For trenches in carriageways or other paved areas the backfill shall be brought up to formation level, or sub-formation level where capping is required, unless a lower level is described in Appendix 5/1.

3 Trenches for Filter drains shall be backfilled as described in Appendix 5/1 and in accordance with the NRA Road Construction Details listed in Appendix 5/7. Filter materials for backfilling shall be selected from Table 5/5 and shall comply with the following:

(i) Hardness (Los Angeles Coefficient): Shape (flakiness index): geometrical requirements in accordance with Table 5/5 and IS EN 13242;

(ii) Resistance to fragmentation in Category LAS0 in accordance with IS EN 13242;

(iii) Chemical Resistance: water-soluble sulphate content of less than 0.38% of sulphate (as SO3) when tested in accordance with IS EN 1744-1, Clause 10;

(iv) Plasticity: be non-plastic when tested in accordance with BS 1377: Part 2.

4 The filter material shall be deposited in layers each not exceeding 225mm loose depth and each layer shall be visually assessed in order to confirm adequate compaction of the material and particularly compaction next to the trench walls has been achieved.

Filter Drain Surface Stabilisation using a Geo-Synthetic Grid

5 The inclusion of a geo-synthetic grid shall be specified in accordance with RCD/500/20 at the locations that are likely to be subjected to vehicular overrun. As a minimum geogrid shall be specified at the following locations:

(i) where the hard shoulder or hard strip is less than 2.5m

(ii) lay-bys extended 30m either side of lay-by

(iii) approaches to slip roads extended 50m prior to approach;

(iv) toll plazas.

(v) The geosynthetic grid locks the aggregate particles together forming a stable surface that enables fully loaded heavy goods vehicles (HGVs) to be driven into and out of the drain, reducing the risk of the wheels sinking into the drain and the vehicle becoming stranded.

6 The geo-synthetic grid shall be in accordance with Clause 519 of this series
The Type B backfill shall be stopped approximately 150mm to 200mm below the top of the filter drain. The Type B material shall be lightly compacted in accordance with Sub-clauses 1 and 4 of this clause. Care should be taken not to crush the material as this could result in small pieces of aggregate and a consequent reduction in the effectiveness of the reinforcement. A layer of Type A material should be spread over the compacted surface to provide interlock and the geo-synthetic grid placed over this.

The geo-synthetic grid shall be placed flat over the full width of the drain and shall be between 100mm and 150mm below the surface of the filter drain. The drain shall be brought up to surface level with Type A material as shown on the NRA Road Construction Details listed in Appendix 5/7.

The minimum overlap at joints in the geo-synthetic grid shall be 300mm.

Following filling, the final surface of the drain shall be compacted with a plate compactor with an operating weight less than 150kg to lock the aggregate and reinforcement.
Table 5/5: Grading and geometrical requirements for filter drain material.

<table>
<thead>
<tr>
<th>Allowed Materials</th>
<th>Type A</th>
<th>Type B</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>Crushed rock, crushed gravel, recycled aggregates</td>
<td>Crushed rock, crushed gravel, recycled aggregates, LWA[^2]</td>
</tr>
<tr>
<td>Standard</td>
<td>IS EN 13242</td>
<td>IS EN 13242</td>
</tr>
<tr>
<td>Size, mm</td>
<td>0/20</td>
<td>20/40</td>
</tr>
<tr>
<td>Grading and oversize categories</td>
<td>Gp</td>
<td>Gc 80-20</td>
</tr>
<tr>
<td>Oversize category</td>
<td>OC30</td>
<td>-</td>
</tr>
<tr>
<td>Category for tolerances at mid-size sieves</td>
<td>-</td>
<td>GTNR (no requirement)</td>
</tr>
<tr>
<td>Category for maximum fines</td>
<td>UF3</td>
<td>FnR (no requirement)</td>
</tr>
<tr>
<td>Flakiness Index, FI</td>
<td>FI50</td>
<td>FINR (no requirement)</td>
</tr>
<tr>
<td>LA Abrasion (LA)</td>
<td>-</td>
<td>LA50</td>
</tr>
<tr>
<td>Water Soluble Sulfate (SS) as SO₃%</td>
<td>-</td>
<td>SS0.38</td>
</tr>
<tr>
<td>Plasticity</td>
<td>Non-Plastic</td>
<td></td>
</tr>
</tbody>
</table>

Summary grading requirements

<table>
<thead>
<tr>
<th>Sieve size, mm</th>
<th>Percentage by mass passing</th>
</tr>
</thead>
<tbody>
<tr>
<td>80</td>
<td>100</td>
</tr>
<tr>
<td>63</td>
<td>98-100</td>
</tr>
<tr>
<td>40</td>
<td>80-99</td>
</tr>
<tr>
<td>20</td>
<td>0-20</td>
</tr>
<tr>
<td>10</td>
<td>0-5</td>
</tr>
<tr>
<td>4</td>
<td>30-75</td>
</tr>
<tr>
<td>2</td>
<td>15-65</td>
</tr>
<tr>
<td>0.5</td>
<td>0-35</td>
</tr>
<tr>
<td>0.125</td>
<td>0-4</td>
</tr>
<tr>
<td>0.063</td>
<td>0-3</td>
</tr>
</tbody>
</table>

[^506]: Connecting to Existing Sewers, Drains, Chambers and Channels

Where described in Appendix 5/1, existing sewers and drains shall be extended, connected and jointed to new drains, chambers or channels. All such connections during the construction of the new sewer, drain or other work shall be made in accordance with Clause 507.1[^4] with regard to cast in and articulation pipes and their positions recorded by the Contractor who shall hand to the Employer’s Representative a copy of the record of the connections made the previous day. Where pipe connections are made to existing brick concrete or stone sewers, drains, chambers or channels, the pipes shall be well and tightly built into the concrete, brick or masonry work and be so placed as to discharge at an angle not greater than 60° to the direction of flow of the sewer, drain or channel and with the end of the pipe carefully cut to the necessary angle. Where the connections are between pipe sewers or drains, special connecting pipes shall be laid and jointed as described in Appendix 5/1.

[^2]: LWA may be used as Type B material once they can be shown to fulfil all of the requirements of the specification as set out in Table 5/5
[^3]: Amended as per Amendment No. 1, item 2.
2 Before entering or breaking into an existing sewer or drain, the Contractor shall give notice of his intention to do so to the authority responsible for the pipeline to which the connection is to be made. Risk assessments shall be completed and all work required to be carried out within confined spaces identified for which specific confined space entry procedures and method statements shall be completed and submitted to the Employers Representative prior to work commencing.

3 Existing sewers or drains no longer required shall, as required by Appendix 5/1, be:

(i) sealed with mix ST2 concrete,

(ii) removed and replaced with acceptable fill material deposited in layers and compacted in accordance with the requirements of the 600 Series or

(iii) grouted with a 1:10, cement: PFA mix. The grout shall use the minimum quantity of water to ensure the fluidity necessary to render it capable of being pumped to the ends of the pipe. It shall be used within one hour of mixing but when the mix contains a retarding admixture this time may be extended in accordance with the manufacturer's instructions. The cement shall be CEM I Portland cement complying with IS EN 197 : Part 1 and the pulverised-fuel ash (PFA) with BS 3892: Part 2, fineness to Zone B and sulphate content not exceeding 1.5%.

4 All existing foul and surface water drainage shall be maintained until the permanent drainage for the execution and completion of the Works shall have been executed, completed and functioning satisfactorily including its discharge to approved outfalls.

5 Ground profiles shall at all times be maintained to shed surface water efficiently and directly into the nearest drain and to prevent penetration of water into or below existing pavements.

6 Existing drainage retained in the execution and completion of the Works shall be flushed and cleared of debris and all repairs carried out to ensure drains shall fulfil the requirements of the Contract. The clearing and repair of any existing drainage shall not have any negative impact on local watercourses.

7 Prior to any connection to the existing drainage system, the Contractor shall carry out a CCTV survey, to the requirements of Series 2900 of the Specification, of the existing system for a minimum length of 300 metres downstream of the connection. The Contractor shall submit a report on the condition of the existing pipe network to the Employer's Representative for Approval, prior to the commencement of Construction of any Works.

507 Chambers for Drains

1 Chambers for Drains shall include manholes, catchpits, inspection chambers and walled soakaways. Chambers shall be of the type specified in Appendix 5/1, constructed in accordance with the NRA Road Construction Details listed in Appendix 5/7 as appropriate to that type and identified as chambers being within the carriageway or chambers located elsewhere, e.g. verge. Access chambers shall have an internal diameter not less than 1050 millimetres. All ST concrete referred to in this Clause shall comply with the requirements of Concrete for Ancillary Purposes in the 2600 Series unless otherwise described in Appendix 5/1.

2 Foundations to chambers shall be of mix ST4 concrete. Channels for chambers shall be formed and finished smooth in the foundation concrete or constructed of preformed half circle channels, with sides benched in mix ST4 concrete, or mortar designation (i) complying with the requirements of the 2400 Series excluding lime.
3 Brickwork and Blockwork shall comply with the 2400 Series and be built with mortar designation (i) in English bond. The joints of brickwork where exposed shall be finished as specified for unpointed joints in the 2400 Series. The ends of all pipes shall be neatly built into the brickwork and finished flush with mortar designation (i).

4 Precast concrete chambers and riser sections shall comply with BS 5911-3, IS EN 1917, IS 420 and the particular requirements described in Appendix 5/1. Cast in situ concrete chambers shall be constructed of mix ST4 concrete complying with Clause 2602 and the particular requirements described in Appendix 5/1.

5 Access ladders and/or step irons, shall only be provided from the first intermediate landing slab down to pipe invert for Type E chambers. Hand/fotholds shall be provided for access into all pipe inverts from the benching for large diameter pipes as shown in the Road Construction Details. Manhole steps complying with IS EN 13101 shall be built in accordance with the NRA Road Construction Details listed in Appendix 5/7. Steelwork used for ladders, handholds and other fittings shall comply with IS EN 10084 and be galvanised in compliance with the requirements of the 1900 Series after fabrication. Threaded components shall also be galvanised in compliance with the requirements of the 1900 Series. Glass Reinforced Plastic used for ladders and step irons shall conform to EN13706 for structural pultruded profiles.

6 Excavation around chambers, except those described in Sub-clause 6 of this Clause, shall be backfilled with general fill material as described in Table 6/1 and compacted in compliance with the requirements of the 600 Series. Where mechanical compaction is impracticable, the excavation shall be backfilled with mix ST2 concrete. Where there are precast concrete access shafts to precast concrete chambers, the shafts shall be surrounded by a minimum thickness of 150 mm of mix ST4 concrete, and the remaining excavation backfilled with general fill material as described in Table 6/1 compacted in compliance with Clause 612.

7 Chamber covers, gratings and frames shall be as described in Appendix 5/1 and shall comply with IS EN 124 and Sub-clauses 7 to 14 of this Clause. Gratings and frames are required to be locked in accordance with Clause 508.4, the minimum size of the locking bolt shall be M16 and it shall be manufactured from stainless steel to prevent corrosion.

8 Permitted Classes of cover and frame shall comply with IS EN124 from the following:

(i) Group 1: Class A15 Pedestrian Traffic Only

(ii) Group 2: Class B125 Footways, pedestrian areas

(iii) Group 3: Class C250 Gullies in road side channels

(iv) Group 4: Class D400 Carriageway, hard shoulders, hardstrips, car parks

Class D400 units shall incorporate a permanent non-rock feature either triangular point suspension, machined faces or cushioned or wedged seating.

9 Permitted clear access for openings shall be either:

(i) Minimum 600mm diameter circular access cover and frame for general use,

(ii) Minimum 750mm or 900mm circular access cover and frame where required for chambers deeper than 2m as specified in Appendix 5/1

10 Requirements for special duty covers for use in carriageways in accordance with IS EN124 Group 5: Class E600 or Group 6: Class F900 shall be as described in Appendix 5/1.
11 Gratings for catch-pit chambers shall have a minimum waterway area as described in Appendix 5/1.

12 Two sets of lifting keys shall be delivered to the Employer's Representative for each type of cover supplied. At least two keyways shall be provided in each complete cover, one in each segment for segmental covers. A recess for a prising bar shall be incorporated in manhole covers unless other means of loosening the cover from the frame are provided.

13 Frames for chamber covers and gratings shall be set in cement mortar designation (i) complying with Clause 2404 or a proprietary quick setting mortar of equivalent strength.

Covers and gratings located within the carriageway, hardshoulder or hardstrip shall be set in a mortar with the following properties:

(i) The material shall be non-shrink

(ii) The material shall have a minimum workable life of 15 minutes

(iii) The compressive strength of the material shall exceed 30N/mm² in 3 hours

(iv) The tensile strength of the material shall exceed 5N/mm² in 3 hours.

Frames shall be identified in Appendix 5/1 as being Plain Seated where unsealed or either single or double seal when required to be sealed. Covers shall be specified in Appendix 5/1 as being: Watertight, Pressure Tight or Ventilated

14 For all pipelines the nearest joint to any chamber shall be not more than 500mm from the inner face of the wall and shall not be restricted by any concrete. Between this and the next joint, the length of the articulated pipe shall be in accordance with Table 5/6 and as shown on Road Construction Detail RCD/500/14

Table 5/6: Length of Articulated ‘(Rocker)’ Pipe.

<table>
<thead>
<tr>
<th>Nominal Pipe Diameter (mm)</th>
<th>Length of Pipe ‘Y’ (mm)</th>
</tr>
</thead>
<tbody>
<tr>
<td>450 and less</td>
<td>500 to 750</td>
</tr>
<tr>
<td>Greater than 450</td>
<td>750 to 1000</td>
</tr>
</tbody>
</table>

15 Where the adjustment or replacement of existing chamber frames and covers is required, the units shall be taken up and refixed or replaced with new units complying with Sub-clauses 9 to 15 of this Clause or such other specification as required by the authority responsible for the utility to which the cover provides access. The frames and covers or gratings shall be laid on a mortar bed complying with Sub-clause 13 of this Clause. The finished thickness of the mortar bed shall be between 10mm and 25mm. Unless otherwise described in Appendix 5/1 covers in grassed verges shall typically be set 20 to 30mm lower than the verge level, and adjusted or replaced chamber frames and covers or gratings shall be set flush with the new surface. Any additional adjustments shall be by modifying the brickwork or precast concrete or insitu concrete riser in compliance with Sub-clauses 507.3 or 507.4 or by using a frame of a suitable depth.

16 Covers in verge locations shall receive a 300mm wide ST4 concrete apron in accordance with National Roads Authority Road Construction Details.

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4 Amended as per Amendment No. 1, item 3.
508 Gullies and Pipe Junctions

1 Permuted gully types shall be either trapped or untrapped and shall be described in Appendix 5/1, and be in accordance with the NRA Road Construction Details listed in Appendix 5/7.

2 Gullies shall be constructed so that no part of the spout or trap has a cross-sectional area less than 2/3rd that of the outlet. The depth of water seal in trapped gullies shall be not less than 50mm.

3 Precast concrete gullies shall comply with BS 5911-6. Insitu concrete gullies shall be constructed of mix ST4 concrete or concrete blockwork of 225mm minimum thickness and be as described in Appendix 5/1.

Where insitu concrete gullies are formed with permanent shuttering, such shuttering shall have a current Irish Agrément Board Roads and Bridges Certificate or equivalent.

4 Cast iron and cast steel gully gratings shall comply with IS EN 124, Sub-clauses 10 to 16 of Clause 507 and shall be in accordance with the NRA Road Construction Details listed in Appendix 5/7. They shall be of the classes and sizes described in Appendix 5/1. All gully gratings shall be fitted with a durable, non-rust locking device.

5 The manufactured upper surface of gully gratings shall be flat unless otherwise described in Appendix 5/1. Slots in gratings or between gratings and frames shall not be orientated parallel to the direction of traffic. Frames shall be bedded on mortar complying with Sub-clause 507.13. Brickwork shall comply with Sub-clause 507.3.

6 Backfilling to precast gullies shall be carried out up to sub-formation level with general fill material Class 1, as described in Table 6/1 compacted in compliance with the requirements of the 600 Series. Where mechanical compaction is impracticable, the backfilling shall be in mix ST4 concrete. The remainder of the backfilling shall be in appropriate capping and road pavement materials except that where mechanical compaction of capping or granular sub-base is impracticable mix ST2 concrete shall be used.

7 Gully connection pipes shall be either flexible or rigid not exceeding 0.7m in length with flexible joints for a distance of 2m from the gully and shall be in accordance with Sub-clause 507.14 when entering chambers. Junction pipes shall be manufactured of the same type and class of material as the remainder of the pipes in the run. Junction pipes which are laid but not immediately connected, shall be fitted with temporary stoppers or seals and the position of all such junctions shall be clearly defined by means of stakes or tracing wires properly marked or labelled. Saddles may be used to form junctions only where permitted in Appendix 5/1. No internal projections greater than 5mm will be permitted. Saddles for plastics pipes shall be installed in accordance with the manufacturer’s recommendations. Saddles with clay pipes shall be jointed with mortar designation (i) complying with Clause 2404, excluding lime. Saddles and pipes shall be surrounded with mix ST2 concrete.

8 Where the adjustment or replacement of existing frames and gratings or kerb type gully covers is required, the units shall be taken up and refixed or replaced with new units complying with Sub-clauses 4 and 5 of this Clause or such other specification required by the Road Authority at a level, unless otherwise described in Appendix 5/1, 6mm below the adjoining road surface on a mortar bed complying with Sub-clause 507.13. The thickness of the mortar bed shall be between

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5 Amended as per Amendment No. 1, item 4.
6 Amended as per Amendment No. 1, item 5.
7 Amended as per Amendment No. 1, item 6.
8 Amended as per Amendment No. 1, item 7.
10mm and 25mm. Any additional adjustment shall be made by modifying the brickwork in compliance with Sub-clause 507.3 or by using a frame of suitable depth.

9 Gullies shall not be located at pedestrian crossings, where practicable. Where gully spacings coincide with pedestrian crossing locations, the nearest gully shall be positioned immediately adjacent to the higher side of the pedestrian crossing. Where a sag point occurs at a pedestrian crossing, a single gully shall be positioned on either side of the crossing point.

509 Testing and Cleaning

1 Drains required in Appendix 5/1 to have watertight joints shall be tested as described in Appendix 1/5 in sections, e.g. between chambers, by means of the air test described in Sub-clause 2 of this Clause. If a pipeline is rejected because of a failed air test, as part of the rectification work, a water test as described in Sub-clause 3 of this Clause may be carried out as an alternative acceptability test. Before testing, the ends of the pipeline to be tested, including those of short branches, shall be plugged and sealed.

2 For the pipeline air test, air shall be pumped in by suitable means until a stable pressure of 100mm head of water is indicated in a U-tube connected to the system. The air pressure shall not fall to less than 75mm head of water during a period of 5 minutes without further pumping, after an initial period to allow stabilisation. Drains with traps shall be tested to 50mm head of water and the permissible loss shall then be no more than 13mm head of water in 5 minutes without further pumping after the initial stabilising period.

3 For the pipeline water test, the pipes shall be filled with water under a head of not less than 1.2m above the crown of the pipe at the high end and not more than 6m above the pipe at the low end. Steeply graded pipelines shall be tested in sections so that the above maximum is not exceeded. The test shall commence two hours after filling the test section at which time the level of water at the vertical feed pipe shall be made up to produce the required 1.2m minimum test head. The loss of water over a 30 minute period shall be measured by adding water at regular 10 minute intervals to restore the original water level and recording the amounts so added. The drain will have passed the test if the volume of water added does not exceed one litre per hour per linear metre of drain per metre of nominal internal diameter.

4 All pipelines less than 350mm diameter, excluding service ducts shall be checked by drawing through each completed length of pipe a spherical mandrel of a diameter 10mm less than the smallest internal pipe diameter permitted by the tolerances specified for the pipes being tested.

5 During the progress of the Works all existing chambers, gullies and rodding eyes shall be kept clean and free from obstruction. On completion of the whole of the Works, all chambers, gullies and drains including verge / surface water drains and filter drains but excluding all fin and narrow filter drains shall be flushed from end to end with water and left free from obstructions. Unless otherwise required in Appendix 5/9 all carrier, foul and filter drains shall be surveyed by Closed Circuit Television (CCTV) in accordance with the relevant requirements of the 2900 Series. Catch-pit chambers shall be left clean and free from silt.

6 The pipes and filter material of filter drains shall at all times be left clean and free from silt and obstruction.

7 The Contractor shall carry out trial holes upon completion of filter drain trenches as directed by the Employer’s Representative to demonstrate that the filter material has not become clogged with other material during construction. If the filter material is clogged then the Contractor shall replace it with clean material.

8 Permeability tests shall be as described in Appendix 5/1.
510 Surface Water Channels, Swales and Drainage Channel Blocks

Insitu Concrete Surface Water Channels

1 Insitu Concrete Surface water channels (CSWC), Grassed Surface Water Channels (GSWC) and drainage channel blocks shall be constructed as described in Appendix 5/3.

2 Insitu Concrete Surface water channels shall comply with the requirements of the 1100 Series for Insitu Channels and NRA Road Construction Detail RCD/500/22 and Clause 510.5 of this Series with regard to intermediate, terminal and weir outlet chambers. Channels shall be either triangular or trapezoidal in cross section.

Precast Concrete Drainage Channel Blocks

3 Drainage channel blocks shall comply with the requirements of the 1100 Series for Precast Channels.

Grassed Surface Water Channels

4 Grassed surface water channels shall be either triangular or trapezoidal in section with a maximum side slope angle of 1(v):5(h). They shall comprise 300mm minimum depth of granular unbound subsoil material in accordance with either Clause 803, 804 or 807. Topsoil shall be placed and seeded with dwarf grass varieties in accordance with Series 600 Clause 618 to a maximum depth of 100mm. Grassed Surface Water Channels shall comply with Clause 510.5 of this Series with regard to intermediate, terminal and weir outlet chambers.

Swales

5 Swales shall be in accordance with National Roads Authority Road Construction Detail RCD/500/25.

Chambers for Surface Water Channels

6 Intermediate, terminal and overflow chambers shall be constructed in accordance with NRA Road Construction Details.
511 Land Drains

1 Existing land drains which are permanently severed by the Works shall be located and connected into a new drain, pipe or ditch all as described in Appendix 5/1. The lengths remaining within the Works shall be cleaned out from the new drain trench face as necessary. Any pipe disturbed by the Works shall be re-laid to ensure a free discharge into the new drain. Disused ends of intercepted land drains be adequately sealed with mix ST2 concrete in compliance with the requirements of the 2600 Series for Concrete for Ancillary Purposes.

2 Where an existing land drain is exposed and severed by temporary trench excavation, the Contractor shall mark the position of the drain and record it. The drain shall be diverted into an existing drain or watercourse. Alternatively, the normal functioning of the drain shall be continued by the construction of a pipeline or channel adequately supported across the excavation, until permanent restoration is made on the original line.

3 The Contractor shall notify the Employer’s Representative of any land drain which is blocked or is otherwise defective when the drain is first exposed.

512 Backfilling to Pipe Bays and Verges on Bridges

1 Unless otherwise described in Appendix 5/1, filling to pipe bays and verges on bridges shall be well graded granular material not exceeding 20mm size containing not more than 3% of material passing the 63 micron sieve and with a uniformity coefficient of more than 5. It shall be laid and compacted in compliance with Clause 505. The material shall meet the sulphate requirement described in Sub-clause 503.3.

Table 5/7: Sampling Rates

<table>
<thead>
<tr>
<th>No of Pipes In Batch</th>
<th>Sample Size</th>
<th>If All Pass</th>
<th>If 2 or more Fail</th>
<th>If 1 Fails take a Further Sample of</th>
<th>If 1 or more of the further Sample Fails</th>
</tr>
</thead>
<tbody>
<tr>
<td>&lt;40</td>
<td>2</td>
<td>ACCEPT</td>
<td></td>
<td>2</td>
<td>REJECT or test ALL pipes in batch, failing pipes to be rejected</td>
</tr>
<tr>
<td>41-60</td>
<td>3</td>
<td></td>
<td></td>
<td>3</td>
<td>REJECT or test ALL remaining pipes in batch, failing pipes to be rejected</td>
</tr>
<tr>
<td>61-80</td>
<td>4</td>
<td></td>
<td></td>
<td>4</td>
<td></td>
</tr>
<tr>
<td>80-100</td>
<td>5</td>
<td></td>
<td></td>
<td>5</td>
<td></td>
</tr>
<tr>
<td>101+</td>
<td>5 per batch of 100</td>
<td></td>
<td></td>
<td>5</td>
<td></td>
</tr>
</tbody>
</table>

ACCEPT or test ALL, failing pipes to be rejected
513 Permeable Backing to Earth Retaining Structures

1 Permeable backing shall consist of one of the following materials:

(i) A minimum thickness of 300 mm of granular material of 20mm nominal size, satisfying the following criteria.

Piping ratio, defined as

\[
\frac{15 \text{ per cent size of the drainage material}}{85 \text{ per cent size of the backfill material}} < 5
\]

Permeability ratio, defined as

\[
\frac{15 \text{ per cent size of the drainage material}}{15 \text{ per cent size of the backfill material}} < 5
\]

where the per cent size of a material is the size of particle corresponding to the given per cent ordinate of the particle size distribution graph.

(ii) Porous no-fines concrete, cast in situ 225 mm thick complying with the requirements of the requirements of the 2600 Series.

(iii) Precast hollow concrete blocks complying with IS EN 772-2 & 3 laid in stretcher bond with dry joints in 225 mm thick walling with holes vertical.

514 Fin Drains

General

1 Fin drains shall comply with this Clause and the special requirements described in Appendix 5/4.

The terms thickness, width, height and core shall have the meanings indicated on NRA Road Construction Details listed in Appendix 5/7 unless otherwise described in Appendix 5/4.

Drain Types 5 and 6 shall be as show on the NRA Road Construction Details listed in Appendix 5/7

The term fin drain shall mean a planar geocomposite structure designed to perform the same function as a narrow filter drain.

2 Where fin drains are designed for lateral entry of water from one side only the requirements for flow rates in Sub-clauses 4 and 5 of this Clause shall apply to the face or plane designed to admit or transmit water.

*Amended as per Amendment No. 1, item 8.*
3 The materials of which the drain is made shall be treated so that they are protected from the deleterious effects of short term exposure to ultraviolet light, and shall be resistant to degradation by acids, alkalis, common chemicals, bacteria, fungi and moulds occurring in soils and road construction materials. After exposure to ultraviolet light the Employer’s Representative may require evidence that the materials still comply with the requirements of this Clause. The drain shall be protected from damage and ultraviolet light and be labelled to identify the grade and manufacturer or supplier.

4 Where necessary, the side intended for entry of water and the direction of in-plane flow shall be identified.

Geotextile

5 The geotextile shall:

(i) in both machine and cross-machine directions, sustain a tensile load of not less than 5.0 N/m at break and have a minimum failure strain of 10% when determined in accordance with IS EN ISO 10319;

(ii) have a minimum puncture resistance of 1200 N when determined in accordance with IS EN ISO 12236;

(iii) have a minimum tear resistance of 200 N when determined in accordance with ASTM Standard D4533-91 (1996);

(iv) have a size distribution of pore openings such that the apparent opening size O90 when determined in accordance with IS EN ISO 12956, or other appropriate test, is as stated in Appendix 5/4;

(v) allow water to flow through it, in either direction, normal to its principal plane at a rate of not less than that stated in Appendix 5/4 under a constant head of water of 100 mm and a maximum breakthrough head of 50 mm when determined in accordance with IS EN ISO 12958.

Composite Drain

6 The composite drain shall:

(i) have a flow rate through each face of the drain of more than 75% of the value specified in Sub-clause 5(v)\(^{10}\) of this Clause on the side or sides where inflow occurs. This value may be found by either:

   (a) direct measurement of the composite drain using IS EN ISO 12958; or

   (b) calculation based on the flow rate obtained by the standard test in IS EN ISO 12958 and the percentage contact area of the drainage core obtained in accordance with Sub-clause 14\(^{11}\) of this Clause;

(ii) have values of long term in-plane flow rates as stated in Appendix 5/4 when determined in accordance with Sub-clauses 14 and 15 of this Clause. The values of hydraulic gradient and minimum applied stresses shall be as given in Table 5/8.

\(^{10}\) Amended as per Amendment No. 1, item 9.
\(^{11}\) Amended as per Amendment No. 1, item 10.
TABLE 5/8: Applied Stresses (kN/m²) and Hydraulic Gradient

<table>
<thead>
<tr>
<th>RCD/500/40 Drain Type</th>
<th>Drain Type 5</th>
<th>Drain Type 6</th>
<th>Drain Type 7</th>
</tr>
</thead>
<tbody>
<tr>
<td>Sub-clause 14: Normal Stress</td>
<td>50</td>
<td>50</td>
<td>50</td>
</tr>
<tr>
<td>Shear Stress</td>
<td>10</td>
<td>10</td>
<td>10</td>
</tr>
<tr>
<td>Sub-clause 15: Normal Stress</td>
<td>100</td>
<td>100</td>
<td>100</td>
</tr>
<tr>
<td>Sub-clause 15: Hydraulic Gradient</td>
<td>0.1 and 1.0</td>
<td>1.0</td>
<td>1.0</td>
</tr>
</tbody>
</table>

Joints

7 Fin drains shall be capable of being jointed longitudinally or laterally into pipe systems or chambers for inflow and outflow purposes and be self-jointing either directly or through purpose made attachment pieces for forming continuous drain lengths. All such joints shall be formed so as to prevent the ingress of soil particles or other extraneous material into the drain.

Fin drain joints transverse to the direction of flow shall have values of in-plane flow rates not less than that required by Sub-clause 5(v)\textsuperscript{12} of this Clause.

Fin drain joints parallel to the direction of flow and any exposed edges shall be protected from the ingress of soil by a geotextile wrapping with a minimum overlap of 150mm.

Pipes

8 For drain Type 6, in accordance with the NRA Road Construction Details listed in Appendix 5/7, pipes shall be perforated or porous and comply with Sub-clause 501.3. Pipes complying with BS 4962: 1989 shall have a minimum Ultimate Pipe Stiffness (STES) value of 1400N/m².

For drain Type 7, in accordance with the NRA Road Construction Details listed in Appendix 5/7, pipes shall be unperforated thermoplastics pipe complying with IS EN 1401, IS EN 1852-1 or IS EN 12666-1 slotted longitudinally along the top surface and stress relief treated (if required) in accordance with the system manufacturer’s published specification.

9 Pipe joints shall comply with the requirements of the relevant European Irish or British Standard for the pipe used or Clause 518.

Backfill and Surround Material

10 Pipe surround material for drain Types 6 and 7, in accordance with the NRA Road Construction Details listed in Appendix 5/7, shall comply with Sub-clause 503.3(i) or 503.3(ii) or Type B material complying with Sub-clause 505.3.

Where fin drains are installed in a trench, backfill material shall be the original as-dug material from the trench unless otherwise specified in Appendix 5/4.

\textsuperscript{12} Amended as per Amendment No. 1, item 11.
Dimensions

11 Unless otherwise described in Appendix 5/4 the dimensions of the fin drain shall be as shown on the NRA Road Construction Details listed in Appendix 5/7. The pipe diameter shall be as stated in Appendix 5/4. The drain slope angle (x) as shown on the NRA Road Construction Details listed in Appendix 5/7 shall be not greater than 15% from the vertical unless otherwise stated in Appendix 5/4.

Installation and Handling

12 Installation of fin drains shall be as shown on the NRA Road Construction Details listed in Appendix 5/7. Where fin drains are assembled on site the assembly area shall be clean and dry and free of wind-borne pollutants. Any material which becomes contaminated must be replaced. No geotextile or core material shall be exposed to daylight (or any source of ultraviolet radiation) for a period exceeding a cumulative total of 50 hours. Any geotextile or core material exposed to daylight (or any source of ultraviolet light) for a period exceeding a cumulative total of 50 hours shall be replaced unless it can be demonstrated that the materials of the drain still comply with the requirements of this Clause.

Where fin drains are laid in trench, the trench bottom shall be free of irregularities and to the required levels given in Appendix 5/4. Rock and other hard protuberances shall be removed and any excess cut in the trench bottom filled and compacted back to the required grade with suitable excavated or imported material.

The drain shall be laid with the appropriate face against the side of the trench adjacent to the carriageway and in the appropriate direction. This side of the trench shall have walls sufficiently clean to enable the fin drain to come into close contact with the wall when the trench is backfilled and compacted. Compaction shall be in accordance with Clause 612. Fin drains installed as part of the permanent Works shall be protected from surface water, contamination, and accidental damage during construction.

The fin drain, pipe surround and backfill shall be installed so as to cause no damage to the fin drain. Where any damage does occur, the damaged materials shall be replaced by new material.

After the installation of the fin drain has been completed a marker tape shall be laid approximately 75mm above the fin drain in the position shown on the NRA Road Construction Details listed in Appendix 5/7. The tapes shall be green self-coloured PVC or polythene plastic not less than 0.1 mm thick and 150mm wide.

Identification

13 The Contractor shall obtain and make available the following information for each separate consignment of fin drain delivered to Site:

(i) geotextile and core name, grade/number and mass per unit area;

(ii) names and addresses of system producer, and geotextile, core and pipe manufacturers;

(iii) manufacturing characteristics and constituents of geotextile and core. This shall include composition and type of constituent filaments, threads, fibres, films, tapes and other components;

(iv) consignment number and delivery date;

(v) a copy of the site delivery note.
Test Method for the Percentage Contact Area of Drainage Core

14

(i) The test determines the area of one face of a drainage core which will be in contact with a geotextile filter as a percentage.

(ii) The apparatus required is as follows:

(a) loading device able to apply a compressive load of at least 2kN and having a flat steel base;

(b) flat steel loading plate 200mm x 200mm;

(c) printers ink and roller (or pad);

(d) sheet of thin compressible rubber;

(e) planimeter.

(iii) The test procedure shall be as follows. Cut three representative test specimens 200mm x 200mm (±2mm). Apply ink to one 200mm x 200mm face of a specimen and cover with a sheet of plain paper and a thin compressible rubber sheet. Place the prepared specimen in to the loading device and gradually apply the load of 2kN and maintain for 5 minutes. Release the load and remove the specimen and separate it from the paper. Using the planimeter find the total area of the paper which has received an imprint. Repeat for all specimens.

(iv) The percentage contact area = total area of imprint x 100 area of test specimen.

(v) The report shall include:

(a) reference to this method;

(b) sample identification details;

(c) individual and mean percentage contact areas;

(d) details of any deviation from the specified test procedure.

(vi) Alternative methods of determining the percentage contact area may be employed with the prior approval of the Employer’s Representative.

Test Method for Determining the Thickness of Fin Drains Under Specified Normal and Shear Stresses

15

(i) The test determines the thickness of the fin drain under sustained normal and shear stresses. A long term thickness (at 100,000 hours) is calculated by extrapolation and a short term equivalent normal load which produces the long term thickness is determined.

(ii) The apparatus required is as follows:

(a) a suitable compression testing machine, which shall have a vertical travel at least the nominal thickness of the specimen. It shall be capable of sustaining the necessary loads to within 1% accuracy for the duration of the test;
(b) the compression testing apparatus, which shall include a fixed base plate and parallel moveable top plate with flat steel surface with sufficient friction to permit the development of the required shear forces;

(c) a means of measuring the mean thickness of the specimen to an accuracy of 0.01mm.

As an alternative to (a) and (b), an appropriate inclined plane and kentledge system may be employed to produce the normal and shear loads.

The test procedure shall be as follows:

(iii) Cut six representative specimens of minimum size 100mm x 100mm symmetrically about the core design. Three specimens shall be tested in accordance with (v) below and three in accordance with (vi) below.

(iv) The test specimen shall be placed symmetrically on the base plate and covered by the top plate. The means of measuring thickness shall be attached and the initial thickness measured.

(v) Apply the load smoothly and as quickly as possible to the top plate. The full load (normal and shear) shall be applied in less than 20 seconds and sustained for at least 1000 hours. The applied stresses shall be those given in Table 5/8. At least four measurements of thickness shall be made during each unit of logarithmic time after the first minute. Determine the long term thickness of the specimen as the thickness of the specimen at 1000 hours reduced by 2T where T is the difference in thicknesses of the specimen thickness recorded at 100 hours and 1000 hours. Repeat the test on the two other specimens. The test specimens shall be maintained at a constant temperature of 20°C ± 2°C throughout the test period.

(vi) Apply increasing increments of normal load to the specimen. Determine the short term equivalent load which shall be the load which when applied for a period of 20±5 minutes produces a specimen thickness equal (within an accuracy of ±0.05mm) to the long term thickness of the specimen obtained at (v) above. Repeat the test on the other two specimens.

(vii) The report shall include:

(a) a reference to this method;
(b) sample identification details;
(c) the initial thickness of the sample;
(d) the applied load;
(e) the thickness of each sample at 100 and 1000 hours and the mean of the three results;
(f) a plot of percentage reduction in thickness against logarithmic time;
(g) the mean long term thickness;
(h) the mean short term equivalent load;
(i) any deviations from the specified test procedure.
Determining In-plane Flow Under Compressive Loading

16 In-plane flow shall be determined in accordance with IS EN ISO 12958 except that the following conditions shall apply:

(a) the applied normal stress shall be the greater of the value given in Table 5/8 (for Sub-clause 15) or the mean short term equivalent stress as determined in Sub-clause 14\(^{13}\) of this Clause;

(b) the sample shall be tested such that the measured flow (or flows) is in the same direction as the principal flow (or flows) when the fin drain is in service;

(c) the foam rubber option of the test procedure shall be used (details of the foam rubber to be used may be obtained from the Employer’s Representative);

(d) the hydraulic transmissivity shall be reported for each of the hydraulic gradients employed.

Test Methods

17 Notwithstanding the requirements of Sub-clauses 14, 15 and 16\(^{14}\) of this Clause, variations in the test methods specified therein shall be made where deemed necessary by the Irish Agrément Board following consultation with the manufacturer. All such variations shall be recorded in the report. Alternatively the manufacturers published technical data and testing information for these parameters may be used.

Certification

18 Fin drains and constituent materials shall have a current Irish Agrément Board Certificate for Roads and Bridges or equivalent certifying the appropriate physical properties when tested in accordance with this Clause.

515 Narrow Filter Drains

General

1 Narrow filter drains shall comply with this Clause and the special requirements described in Appendix 5/4.

The term narrow filter drain refers to drain Types 8 or 9 indicated in NRA Road Construction Details listed in Appendix 5/7. They consist of a porous or perforated pipe laid in a narrow trench surrounded by granular material where the granular material and/or the pipe is enclosed by a layer of geotextile filter. Narrow filter drains and fin drains perform the same function.

\(^{13}\) Amended as per Amendment No. 1, item 12.

\(^{14}\) Amended as per Amendment No. 1, item 13.
Materials

2 The geotextile materials used in the drain shall be stored so that they are protected from the deleterious effects of short term exposure to ultraviolet light, and shall be resistant to degradation by acids, alkalis, common chemicals, bacteria, fungi and moulds occurring in soils and road construction materials.

After exposure to ultraviolet light the Employer’s Representative may require evidence that the materials still comply with the requirements of this Clause. They shall be protected from damage and ultraviolet light and be labelled to identify the grade and manufacturer or supplier.

3 The geotextile used in narrow filter drains shall comply with all requirements of Sub-clause 514.5\(^{15}\) for geotextiles used in fin drains.

For drain Type 8, in accordance with the NRA Road Construction Details listed in Appendix 5/7, the geotextile surround to the pipe shall consist of a prefabricated continuous close fitting sock. Alternatively the pipe shall be firmly wrapped in a single layer of geotextile with an overlap of between 50 and 75 mm and secured around the pipe in a manner so as to prevent the ingress of soil particles or other extraneous material and without affecting the permeability of the wrapped material. Splices between lengths of sock or layer shall have overlaps within these dimensions and be securely tied.

For drain Type 9, in accordance with the NRA Road Construction Details listed in Appendix 5/7, the geotextile surround to the granular material shall have a minimum overlap of 250mm including 100mm down-tuck. Splicing of lengths of geotextile shall consist of minimum 600mm overlap secured with pins or mechanical ties. Where an outlet pipe passes through the geotextile a separate piece of geotextile shall be wrapped round the outlet pipe, flared against the geotextile in the filter drain and secured. Where drain lengths are terminated at chambers, the geotextile shall be secured against the chamber walls by suitable means so as to prevent the ingress of soil particles or other extraneous material into the drain.

4 Pipes and fittings shall comply with Sub-clauses 514.7 and 514.8. Where coilable pipes to BS 4962:1989 are used they shall be capable of being straightened so as to lie flat without restraint in the trench bottom before backfilling.

5 The granular material used for trench infill shall comply with the requirements for non-plasticity, LA category and sulphate content of Sub-clause 505.3 and have a grading within the limits of Table 5/9. The material when tested in accordance with Sub-clause 509.8\(^{16}\) shall meet the permeability requirements described in Appendix 5/4.

\(^{15}\) Amended as per Amendment No. 1, item 14.
\(^{16}\) Amended as per Amendment No. 1, item 15.
TABLE 5/9: Narrow Filter Drain: Trench Infill Grading Requirements

<table>
<thead>
<tr>
<th>RCD Drain Type</th>
<th>Type 8</th>
<th>Type 9</th>
</tr>
</thead>
<tbody>
<tr>
<td>Maximum Particle Size mm</td>
<td>40</td>
<td>40</td>
</tr>
<tr>
<td>D5 Size mm</td>
<td>&gt;0.125</td>
<td>&gt;0.125</td>
</tr>
<tr>
<td>D15 Size mm</td>
<td>As stated in Appendix 5/4</td>
<td></td>
</tr>
<tr>
<td>D85 Size mm</td>
<td>-</td>
<td>&gt; Hole Diameter or 1.2 Times Width of Slot in Pipe</td>
</tr>
<tr>
<td>Uniformity Coefficient (C_u) D60/D10</td>
<td>5 to 12</td>
<td>5 to 12 or if C_u&lt;5 D5&gt;1mm</td>
</tr>
</tbody>
</table>

Dn = Sieve size passing n% by weight of material

Dimensions

6 Unless otherwise described in Appendix 5/4 the dimensions of the narrow filter drain shall be as shown on the NRA Road Construction Details listed in Appendix 5/7. The pipe diameter shall be as stated in Appendix 5/4. The drain slope angle (x), as shown on the NRA Road Construction Details listed in Appendix 5/7, shall be not greater than 15% from the vertical unless otherwise stated in Appendix 5/4.

Installation and Handling

7 Narrow filter drains shall be installed as shown on the NRA Road Construction Details listed in Appendix 5/7. Before during and after installation the geotextile shall be protected from contamination, damage and exposure to ultraviolet radiation in accordance with Sub-clause 514.3.

The excavated trench bottom shall be free of irregularities and to the required levels given in Appendix 5/4. Rock and other hard protuberances shall be removed and any excess cut in the trench bottom filled and compacted back to the required grade with suitable excavated or imported material. Drain Type 9, in accordance with the NRA Road Construction Details listed in Appendix 5/7, shall have trench walls sufficiently clean to enable the geotextile to come into close contact with the wall when the granular material is placed inside it. The deposition and compaction of infill shall be in accordance with Sub-clause 505.4 for filter drains.

Narrow filter drains installed as part of the permanent Works shall not be used for the disposal of surface water run-off during construction. Narrow filter drains exposed to surface water ingress shall be temporarily protected.

8 The Contractor shall obtain and make available the information required in Sub-clause 514.13 in respect of the geotextile and pipe type used in narrow filter drains.

17 Amended as per Amendment No. 1, item 16.
Certification

9 The geotextile shall have a current Irish Agrément board Certificate certifying the appropriate physical properties when tested in accordance with this Clause.

516 Combined Drainage and Kerb Systems

1 The Contractor shall design the combined drainage and kerb systems listed in Appendix 1/11 in accordance with IS EN 1433 and the design requirements given in Appendix 5/5.

2 Combined drainage and kerb systems shall permit lateral entry of surface water from the channel either continuously or at intervals not exceeding 1m.

3 Combined drainage and kerb systems shall be suitable for their intended application and place of installation in the Works. The Contractor shall submit evidence of such suitability for purpose to the Employer's Representative.

4 Proprietary systems shall have a current Irish Agrément Board Roads and Bridges Certificate (or equivalent) and shall be laid and jointed in accordance with the manufacturer’s written instructions.

5 Design flows given in Appendix 5/5 shall be accommodated without surcharge within the main combined drainage and kerb system and beneath the underside of any inlet slot sections. Combined drainage and kerb systems shall be designed to ensure a self-cleansing velocity of 0.75m/s in the design flow.

6 Where applicable, the width and depth of the combined drainage and kerb system units shall not exceed the dimensions given in Appendix 5/5.

7 When used adjacent to porous asphalt surfacing materials, units shall incorporate side entry inlets to permit drainage of water held within the porous asphalt. Inlets shall comply with the requirements of Appendix 5/5 and shall have the capacity to drain porous asphalt.

8 Combined drainage and kerb systems, excluding insitu system, shall be classified as follows according to their intended use and shall be as stated in Appendix 5/5:

   (i)  C250; or
   (ii) D400; or
   (iii) In exceptional circumstances where combined kerb and drainage units are to be located in areas subjected to large numbers of high speed heavy goods vehicles, Class E600 combined kerb and drainage units shall be considered.

9 Adjacent carriageway, footway, verge or central reserve construction shall not take place until a visual inspection has been completed by the designer regarding: bedding, backing, surrounding or jointing of combined drainage and kerb units. The visual assessment shall be supported by an assessment of the concrete suppliers data presented as a graph showing strength gain against time for the particular concrete mix.

10 Joints between units and between the channel and units, shall be designed to avoid leakage of surface water. Joints between bridge deck waterproofing and component parts passing through the waterproofing shall be watertight. Sealants shall be compatible with the waterproofing system.
11 Junctions, connecting pipes and other fittings comprising the combined drainage and kerb system shall comply with Sub-clause 501.3 and shall be as described in Appendix 5/5. The combined drainage and kerb system shall incorporate measures to enable rodding of the outfall pipework and adequate access for jetting by water jetting equipment of the system.

12 The system shall be cleaned out by high pressure water jetting or other appropriate means on completion of the Works. The system shall be left clean and free from all obstruction.

517 Linear Drainage Channel Systems

General

1 The linear drainage channel systems listed in Appendix 1/11 shall be treated as other features to be designed by the Contractor. The insitu linear drainage channel systems shall conform, where applicable, to Sub-clauses 517.2 to 517.17 and the requirements in Appendix 5/6 and Appendix 1/11 as appropriate. The prefabricated linear drainage channel systems, where applicable, shall conform to IS EN 1433 and with the other requirements in Appendix 1/5.

2 Linear drainage channel systems shall be suitable for their intended use and place of installation in the Works. The Contractor shall submit evidence of such suitability to the Employers Representative in accordance with Appendix 1/5.

3 Design flows given in Appendix 5/6 shall be accommodated without surcharge within the main channel section and beneath the underside of any inlet slot sections.

4 Where applicable, the width and depth of linear drainage channel system units shall not exceed the dimensions given in Appendix 5/6.

5 Dimensions of inlet slots shall comply with IS EN 1433.

6 When used adjacent to porous asphalt surfacing materials, units shall also incorporate side entry inlets to permit drainage of water held within the porous asphalt. Inlets shall comply with the requirements of Appendix 5/6 and shall have the capacity to drain porous asphalt.

Weathering Resistance

7 Linear drainage channel systems shall be marked in accordance with IS EN 1433 according to the grade of weathering resistance for drainage channels as stated in Appendix 5/6.

Classification

8 Linear drainage channel systems shall be classified as follows according to their intended use and shall be as stated in Appendix 5/6:

(i) C 250; or

(ii) D 400; or

(iii) In exceptional circumstances where linear drainage channel systems have to be located in areas subjected to large numbers of high speed heavy goods vehicles, Class E 600 linear drainage channel systems shall be considered.
Watertightness

9 Joints between units comprising the system and between adjacent construction and the system shall be designed to avoid leakage of surface water. Where applicable, joints between bridge deck waterproofing and component parts passing through the waterproofing shall be watertight. Sealants shall also be compatible with the waterproofing system.

10 Junctions, connecting pipes and other fittings comprising the linear drainage system shall comply with the Sub-clause 501.3 and shall be as described in Appendix 5/6. The linear drainage channel system shall incorporate measures to enable rodding of the outfall pipework and adequate access for jetting by water jetting equipment into the system.

Cleaning

11 The system shall be cleaned out by high pressure water jetting or other appropriate means on completion of the Works. The system shall be left clean and free from all obstruction.

Manufactured Systems

12 Manufactured systems shall conform to IS EN 1433 and to the following:

(i) Products shall be designed and developed within an IS EN ISO 9001 system that has been assessed by a relevant recognised certification body. A relevant certification body shall have IS EN 45011 and IS EN ISO/IEC 17021 within its scope.

(ii) Products shall be type tested by a third party testing laboratory/organisation that has been accredited to IS EN ISO/IEC 17021 and has IS EN1433 within its scope.

(iii) The certification process shall be overseen or audited by a single relevant accredited certification body meeting the requirements in (i) and (ii) above.

(iv) Copies of all certificates shall be forwarded to the Employers Representative along with audited product test reports.

(v) All documentation must be furnished in English or Irish.

Insitu Systems

13 Insitu systems shall comply with the requirements of the 1100 Series except that the concrete shall be considered as plain concrete within the terms of this Sub-clause, irrespective of the inclusion of reinforcement.

14 Insitu systems shall be slip formed except for sections at gullies which shall be hand formed. Full depth joints shall be constructed at the interface at each side of gullies with a 25mm thick filler board complying with the requirements of the 1000 Series for Joint Filler Board. Joints shall be prepared and sealed in accordance with the 1000 Series.

15 The central void may be formed by an inflated tube which is then removed, or by an insitu suitable pipe or similar former fit for the purpose. At gully positions the inner former shall be pre-sleeved with a suitable pipe of similar former fit for the purpose which shall be of sufficient length to overlap the two joins on either side of the gully.

16 The central void shall be checked in accordance with Sub-clause 509.4. Verification of slot widths shall be determined by suitable templates.
17. Reinforcement shall comply with the 1700 Series. Concrete cover shall be not less than 75mm.

18. Trial lengths and testing of insitu channels shall be undertaken in accordance with Clauses 6 and 8.6 of BS 5931 and the relevant paragraphs of Clause 9 of IS EN 1433 respectively.

518 Thermoplastics Structured Wall Pipes and Fittings

General

1. Thermoplastics structured wall pipe shall comply with this Clause and the special requirements described in Appendix 5/1. The term structured wall pipe shall mean all types of smooth bore pipe except solid wall homogeneous pipe. Typical forms of construction classified as structured wall pipes include: single wall externally structured smooth bore, twin wall, foamed core and spirally wound. The term fitting shall mean a product used in conjunction with the pipe to form the system but excluding gullies manhole chambers, inspection chambers and access chambers.

Materials

2. The materials from which the pipe and fittings are made, shall be treated so that they are protected from the deleterious effects of short term exposure to ultraviolet light, and shall be resistant to degradation by acids, alkalis, common chemicals, bacteria, fungi and moulds occurring in soil, road construction materials and road drainage systems. In addition, the materials from which the pipe and fittings are made, shall not incorporate any additives in quantities sufficient to cause microbiological degradation or to impair the conformity to the chemical, physical and mechanical properties or impact resistance requirements given in Sub-clause 5 of this Clause.

The specification of the raw material shall be agreed between the certification body, as defined in Sub-clause 15 of this Clause, and the manufacturer and may incorporate re-processable and/or recyclable material. The agreed specification shall incorporate tolerances for each of the relevant characteristics defined in the appropriate clause of IS EN 13476. The Contractor shall submit, prior to commencement of the Works, completed information sheets in accordance with Appendix 5/8.

Dimensions

3. Systems for carrier drainage shall be between 150mm and 900mm nominal internal diameter. Pipes for narrow filter drains shall be 110mm or 150mm nominal internal diameter. Pipes for sub-soil drainage shall incorporate slots or holes with a minimum cross sectional area of 1000 mm$^2$ per metre run of pipe. The bore of the pipe and fittings shall be in accordance with the standard tolerances for nominal bores given in IS EN 476.

Appearance

4. The system shall have a smooth internal bore and be free from any burs, flash or other inconsistencies that could have a detrimental effect on the performance of the system. Pipes and fittings for drainage shall be externally coloured either terracotta or black.

Structured Wall Pipe

5. The structured wall pipe shall have the properties defined in Table 5/10
### TABLE 5/10: Requirements for Structured Wall Pipe

<table>
<thead>
<tr>
<th>Property</th>
<th>Relevant Standard</th>
<th>Requirement</th>
</tr>
</thead>
<tbody>
<tr>
<td>General</td>
<td>IS EN ISO 2897-1, IS EN ISO 2897-2 and ISO 11922-1</td>
<td>Dimensions to be specified</td>
</tr>
<tr>
<td>Ring stiffness</td>
<td>IS EN ISO 9969</td>
<td>6kN/m² minimum.</td>
</tr>
<tr>
<td>Creep Ratio</td>
<td>IS EN ISO 9967</td>
<td>PVC-U - maximum 2.5 PP and PE - maximum 4.0</td>
</tr>
<tr>
<td>High volume low pressure jetting</td>
<td>Sewer Jetting Code of Practice published by Water Research Centre</td>
<td>Minimum acceptable failure pressure 137 bar.</td>
</tr>
<tr>
<td></td>
<td>Frankland Road, Blagrove, Swindon, SN5 8YF, UK.</td>
<td></td>
</tr>
<tr>
<td>Longitudinal bending</td>
<td>Sub-clause 520.1^{18}</td>
<td>Pipes with nominal diameters ≤ 350mm to have a difference in dimensions when measured in the vertical axis of less than 5% of the pipe length and no local permanent deformation occurs during the test.</td>
</tr>
<tr>
<td>Impact resistance at 0°C</td>
<td>IS EN 1411 with d25 striker of 1 kg</td>
<td>Preliminary test – test 10 pieces as described in IS EN 1411, dropping the striker from a height of 1m. If any test pieces fail, subject the pipe to the full test given in Clause 7.3 of IS EN 1411 starting the striker from a drop height of 400mm. The mean minus 1.64 times the standard deviation must exceed 1m.</td>
</tr>
<tr>
<td>Impact resistance at 23°C</td>
<td>IS EN 1411 with striker as above</td>
<td>Value to be derived from the same batch of pipe as used in the impact resistance test at 0°C. The H50 (mean) value - 10% to be used as the minimum value for quality control testing. Alternatively the 0°C test can be used as a QC test if the manufacturer chooses.</td>
</tr>
<tr>
<td>Rodding resistance</td>
<td>Sub-clause 520.1^{19}</td>
<td>Pipes with nominal diameters ≤ 350mm to have an average failure energy &gt;3 joules</td>
</tr>
<tr>
<td>Static friction coefficient (ducts)</td>
<td>ESI (Electricity Supply Industry) 12-24 Test TT3</td>
<td>Pass</td>
</tr>
<tr>
<td>Creep at elevated temperature</td>
<td>IS EN ISO 9967</td>
<td>Test to be carried out at 45°C, creep ratio to be less than 2 times the (ducts) values to IS EN ISO 9967.</td>
</tr>
<tr>
<td>Resistance to point loads (ducts)</td>
<td>Sub-clause 520.1^{20}</td>
<td>No perforation at 10% rod travel</td>
</tr>
<tr>
<td>Tensile strength of a seam</td>
<td>IS EN 1979</td>
<td>IS EN 13476</td>
</tr>
</tbody>
</table>

### Fittings

6 The fittings for use with structured wall pipe shall have the properties defined in Table 5/11.

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18 Amended as per Amendment No. 1, item 17.
19 Amended as per Amendment No. 1, item 18.
20 Amended as per Amendment No. 1, item 19.
TABLE 5/11: Requirements for Fittings

<table>
<thead>
<tr>
<th>Property</th>
<th>Relevant Standard</th>
<th>Requirement</th>
</tr>
</thead>
<tbody>
<tr>
<td>General</td>
<td>IS EN ISO 2897-1, IS EN ISO 2897-2 and ISO 11922-1</td>
<td>Dimensions to be specified</td>
</tr>
<tr>
<td>Ring stiffness (excluding couplers)</td>
<td>ISO/DIS 13967</td>
<td>6kN/m² minimum</td>
</tr>
<tr>
<td>Rodding resistance</td>
<td>Sub-clause 520.12</td>
<td>Fittings with nominal diameters ≤ 350mm to have an average failure energy &gt;3 joules</td>
</tr>
<tr>
<td>Strength and flexibility of fabricated fittings</td>
<td>IS EN 12256</td>
<td>IS EN 12256</td>
</tr>
<tr>
<td>Impact resistance (drop test)</td>
<td>Drop Test to IS EN 12061: 1999</td>
<td>Fall height 1000mm at a temperature of 0°C. Product less than ND 300 shall show ‘no damage’. Others may fail but must be identified as ‘handle with care’</td>
</tr>
<tr>
<td>Watertightness of fabricated fittings</td>
<td>IS EN 1053</td>
<td>0.5 bar for 1 minute</td>
</tr>
</tbody>
</table>

Pipe and Fittings

7 The pipe and fittings shall have the properties defined in Table 5/12.

TABLE 5/12 Requirements of the Systems

<table>
<thead>
<tr>
<th>Property</th>
<th>Relevant Standard</th>
<th>Requirement</th>
</tr>
</thead>
<tbody>
<tr>
<td>Leak-tightness of joints – diameter distortion (watertight joints)</td>
<td>IS EN 1277 Method 4 Condition B Temperature (23 ± 2°C)</td>
<td>Use default values from ISEN 1277</td>
</tr>
<tr>
<td>Leak-tightness of joints – angular deflection (watertight joints)</td>
<td>IS EN 1277 Method 4 Condition C Temperature (23 ± 2°C)</td>
<td>Use default values from IS EN 1277</td>
</tr>
<tr>
<td>Leakage rate from partially-watertight joints</td>
<td>Sub-clause 509.7</td>
<td>Less than 20 times the square of the internal diameter of the pipe in metres shall flow through the joint in litres per minute</td>
</tr>
<tr>
<td>Resistance to wheel loads</td>
<td>IS EN 1437 adapted to suit NRA loading conditions</td>
<td>Less than 5% deformation when loaded to 100kN (for unequal branches only)</td>
</tr>
</tbody>
</table>

Bedding, Backfill and Surround Material

8 All systems shall be installed in accordance with the pipe and bedding combinations given in NRA HD 140. Other combinations shall be supported by calculations in accordance with IS EN 1295-1. Bedding, backfill and surround materials are classified in Clause 503 and Clause 505.

Installation and Handling

9 The bedding, surround and backfill shall be installed so as to cause no damage to the pipes and fittings. Installation of the pipe and fittings, particularly, procedures for preparation and execution of jointing operations, shall be in accordance with the manufacturer’s instructions.
Identification

10 The Contractor shall obtain and make available records with the following information for each separate consignment of structured wall pipe or fittings delivered to Site:

(i) system name, ring stiffness grade / number and size;
(ii) name and address of the system manufacturer;
(iii) consignment number and delivery date; and
(iv) a copy of the site delivery note.

Test Method for Longitudinal Bending

11

(i) The test specimen shall be a six metre length of pipe or the maximum length available from the manufacturer (if less than six metres).

(ii) The apparatus shall include:

(a) two level support blocks at least 250mm wide and of sufficient height to allow the pipe to sag over its length without touching the ground;

(b) a means of measuring the vertical distance between the pipe at the centre of the span and a fixed point of reference to an accuracy of ±0.5mm.

(iii) The test procedure shall be as follows:

(a) condition the specimen for at least 1 hour at 23°C ± 2°C;

(b) set the supports at a distance apart equal to the length of pipe minus 500mm;

(c) place the pipe symmetrically on the supports;

(d) measure the distance between the top of the supports and the fixed point of reference, in the vertical axis through the centre line of the pipe;

(e) after a period of two minutes measure the distance between the underside of the pipe at mid span and the fixed point of reference;

(f) record the difference in readings as a percentage of pipe length.

Test Method for Rodding Resistance (Internal Puncture)

12

(i) The test specimens shall be:

(a) twenty specimens cut from the structured wall pipe, each specimen to be 242mm to 246mm in length and a quarter section of the circumference or;

(b) twenty specimens cut from a number of identical fittings.

(ii) The apparatus shall include:
(a) a standard drop weight pipe testing apparatus capable of dropping a tup from a height of 1m;

(b) a 300g tup which can be varied in 30g multiples with a striker consisting a steel rod 18mm in diameter with a 9mm hemispherical end;

(c) a 250mm x 250mm box containing dry Leighton Buzzard sand (Garside quarry) such that there is at least 100mm of sand beneath the test specimen when bedded down.

(iii) Procedure shall be as follows:

(a) mark the intended point on the inside of the pipe on the intersection of the centre lines. With profiled pipes the point of impact shall be at the point nearest the intersection that is midway between the ribs or in the middle of a hollow corrugation;

(b) alternatively mark the intended point of impact on the section of fitting;

(c) condition the test specimens in air at a temperature of 4°C ± 2°C for a period of at least one hour before the test;

(d) locate the box under the drop tube of the impact apparatus;

(e) embed the specimen into the sand using a vibrator with the inner surface exposed and facing upwards;

(f) locate the specimen such that the marked point of impact is under the centre line of the tup;

(g) drop the tup from a height of 1m on to the inner surface of the specimen within 10 seconds of removing the specimen from the conditioning environment;

(h) examine the specimen for damage. Damage is defined as a puncture or crack of the inner layer. Ductile bending or “whitening” is not considered to be damage;

(i) if the specimen exhibits no damage the next test shall be conducted with a tup of 30g greater mass. If the specimen does exhibit damage the next test shall be conducted using a tup of 30g smaller mass;

(j) after having completed the 20 strikes calculate the average of the energies where a pass (no damage) was recorded and the average of the energies where a failure (damage) was recorded, then calculate the average of the two averages.

(iv) The test report shall include:

(a) identification of the samples;

(b) the overall average;

(c) whether or not the specimens were damaged.
Test Method for Resistance to Sharp Objects

13

(i) Test specimens shall be three samples of duct each 300mm long.

(ii) The apparatus shall include:

(a) a compression testing machine;

(b) a 4.7mm diameter steel rod with an hemispherical end;

(c) and a 120° steel vee block at least 300mm long.

(iii) The test method shall be as follows:

(a) condition the test specimens for at least 1 hour at 23°C ± 2°C;

(b) insert the steel rod in the jaws of the moveable platen of the compression testing machine so that the hemispherical end protrudes at least 15% of the nominal pipe diameter;

(c) position the specimen in the vee block and placed directly below the steel rod, for twin-walled pipe the specimen shall be positioned firstly so that the rod strikes on the corrugation or rib, and secondly in the valley (where possible);

(d) set the machine to lower at a rate of 5mm/min;

(e) allow the rod to travel into the pipe a distance equal to 10% of the nominal internal diameter of the pipe (T10mm) or until the pipe wall is perforated. When the outer skin of a twin wall pipe is perforated before 10% is attained, the rod travel up to the point of failure shall be recorded (Tf mm) and the rod allowed to travel to the inner wall. The rod travel shall be continued to a total of 10% (i.e. T10 - Tf) or until complete penetration occurs.

(iv) For each test specimen: pipe size, reference, maximum load and rod travel shall be recorded.

14 Notwithstanding the requirements of Sub-clauses 11, 12 and 13 of this Clause, variations in the test methods specified therein shall be made where deemed necessary by the Irish Agrément Board following consultation with the manufacturer and in agreement with the National Roads Authority. All such variations shall be recorded in the report.

Certification

15 Pipes and fittings shall have a current Irish Agrément Board Roads and Bridges Certificate certifying the appropriate physical properties when tested in accordance with this Clause.
519 Geotextiles, Geosynthetic Clay Liners and Impermeable Membranes

Geotextiles

1 The geotextile materials used in filter drains shall be stored so that they are protected from the deleterious effects of short term exposure to ultraviolet light, and shall be resistant to degradation by acids, alkalis, common chemicals, bacteria, fungi and moulds occurring in soils and road construction materials. After exposure to ultraviolet light the Employer’s Representative may require evidence that the materials still comply with the requirements of this Clause. They shall be protected from damage and ultraviolet light and be labelled to identify the grade and manufacturer or supplier.

2 The geotextile for filter drains shall:

(i) sustain a tensile load of not less than 5.0kN/m at break and have a minimum failure strain of 10% when determined in accordance with IS EN ISO 10319;

(ii) have a minimum puncture resistance of 1200 N when determined in accordance with IS EN ISO 12236;

(iii) have a size distribution of pore openings such that the apparent opening size 090 when determined in accordance with IS EN ISO 12956, or other appropriate test, is less than 300 microns, unless otherwise specified in Appendix 5/4;

(iv) allow water to flow through it, in either direction, normal to its principal plane at a rate of not less than 10 l/m²/s, unless otherwise specified in Appendix 5/4, under a constant head of water of 100mm and a maximum breakthrough head of 50mm when determined in accordance with IS EN ISO 12958.

3 Geotextile joints shall consist of minimum 300mm overlap and shall be secured with pins or mechanical ties where required to ensure lap length is achieved. Where an outlet pipe passes through the geotextile a separate piece of geotextile shall be wrapped round the outlet pipe, flared against the geotextile in the filter drain and secured. Where drain lengths are terminated at chambers, the geotextile shall be secured against the chamber walls by suitable means so as to prevent the ingress of soil particles or other extraneous material into the drain.

Installation and Handling

4 Filter drains shall be installed as shown on the NRA Road Construction Detail listed in Appendix 5/7. Before, during and after installation the geotextile shall be protected from contamination, damage and exposure to ultraviolet21. No geotextile shall be exposed to daylight (or any source of ultraviolet radiation) for a period exceeding a cumulative total of 50 hours. Any geotextile exposed to daylight (or any source of ultraviolet light) for a period exceeding a cumulative total of 50 hours shall be replaced unless it can be demonstrated that the geotextile still complies with the requirements of this Clause.

5 Trench walls shall be sufficiently clean to enable the geotextile to come into close contact with the wall when the granular material is placed inside it. The deposition and compaction of infill shall be in accordance with Sub-clause 505.4 for filter drains.

21 Amended as per Amendment No. 1, item 20.
Geosynthetic Clay Liner

6 Geosynthetic clay liner shall:

(i) Sustain a tensile load of determined in accordance with IS EN ISO 10319;

(ii) Where geosynthetic clay liners and clay liners are proposed within the Works, such liners shall have a maximum permeability of $1 \times 10^{-11}$ metres per second.

Installation and Handling

7 Jointing and splicing: Shall be strictly in accordance with the manufacturer’s instructions

8 Installation: Shall be strictly in accordance with the manufacturer’s instructions

9 Storage: The Geosynthetic Clay Liner shall be stored under cover in clean dry conditions elevated above ground level, protected from mechanical and chemical damage and exposure to extreme temperatures and direct sunlight.

10 Repairs shall be permitted in accordance with the manufacturers requirements. Where the use of bentonite powder is permitted for repairs and jointing it shall be the same granular bentonite as used in the production of the Geosynthetic Clay Liner.

Impermeable Membranes

11 The Contractor shall ensure that all impermeable membranes used in the Works shall be chemically inert and shall not biologically degrade. Such impermeable membranes shall have a minimum life expectancy of 25 years and shall meet the requirement for geotextiles in Appendix C of CIRIA C697.

Installation and Handling

12 Jointing and splicing: Shall be strictly in accordance with the manufacturer’s instructions.

13 Installation: Shall be strictly in accordance with the manufacturer’s instructions.

14 Storage: The membrane shall be stored under cover in clean dry conditions, protected from mechanical and chemical damage and exposure to extreme temperatures and direct sunlight.

15 Repairs: Shall only be allowed strictly in accordance with the manufacturer’s requirements.

Geogrid

16 Geogrid shall:

(i) The geo-synthetic grid shall have either an Irish Agrément Board Roads or Bridges Certificate or British Board of Agrément certification.

(ii) The geo-synthetic grid shall be high density polyethylene or polypropylene, have a nominal ultimate tensile strength in each direction of 30kNm

Installation and Handling

17 Jointing and splicing: Shall be strictly in accordance with the manufacturer’s instructions.
18 Installation: Shall be strictly in accordance with the manufacturer’s instructions: The design drawings shall identify the correct orientation of the geogrids.

19 Storage: The geogrid shall be stored under cover in clean dry conditions, protected from mechanical and chemical damage and exposure to extreme temperatures and direct sunlight.

520 Attenuation

1 Where attenuation ponds are to be provided as part of the design they shall comply with the special requirements described in Appendix 5/9.

2 Proprietary prefabricated buried modular systems shall be constructed and backfilled strictly in accordance with the manufacturer’s instructions.

521 Pollution Control

1 Where pollution control is to be provided as part of the design it shall comply with the special requirements described in Appendix 5/9.

2 Prefabricated Petrol / Oil Interceptors shall be capable of treating design flows stated and shall be identified as being either a full retention or bypass interceptor. They shall be installed and backfilled strictly in accordance with the manufacturer’s instructions.

522 Soakaways

1 Where soakaways are to be provided as part of the design they shall comply with the special requirements described in Appendix 5/9.

2 Where soakaways are being provided, a geotechnical risk assessment shall be undertaken by the Contractor to determine the impact on Structure foundations or embankments in the vicinity of such measures.

3 The Contractor shall provide results for infiltration tests, undertaken as described in BRE Digest 365, at the proposed invert level of all infiltration areas.

4 The invert of soakaways shall not be situated on bedrock and shall be at least 1 metre above the seasonal high groundwater table level.

5 Where Geotextile liner is required it shall comply with the requirements of Clause 519 of this Series.

6 Proprietary prefabricated buried modular systems shall be constructed and backfilled strictly in accordance with the manufacturer’s instructions.

523 Drainage Outfalls

1 Discharge from the drainage systems shall be controlled or attenuated and shall not increase the discharge rate, flow velocity, the risk of erosion or the risk of flooding to any downstream watercourses, pipes, culverts or static water bodies which receive water from an outfall to the extent that the return period for flooding is affected.

2 The Design and Construction of all outfalls shall cause minimum disruption to the existing watercourse at the outfall.
3 The Design and Construction shall provide erosion protection, as shown on National Roads Authority Road Construction Detail RCD/500/50, for all outfall locations of surface water into open watercourses. Material for erosion protection shall be either:

(i) crushed rock in accordance with Series 600, with a grading designed and included within Appendix 5/1; or

(ii) comply with Series 600 for Clause 6B material

(iii) Further details are contained in Road Construction Detail RCD/500/50.

4 Where required by the design Geotextile to Clause 519 shall be identified in Appendix 5/1 and all material shall be placed on a slope not exceeding 1(v):2(h) or benching as stated in Appendix 5/1.

5 Where drainage outfall require non return valves these may include:

(i) flap gates

(ii) positive pressure flexible check valves.

524 Pipes for Ducts

1 Pipes for service ducts, excluding those in use in motorway communications installations, shall be selected from the alternatives in Table 5/2 and shall comply with the standards and particular requirements therein and also where applicable with the requirements of IS EN 61386. Pipes for use in motorway communications installations shall comply with the 1500 series. Pipes for service ducts shall have a smooth internal bore without any sharp edges to the ends of pipes. They shall comply with any additional requirements described in Appendix 5/2, and be of 100mm internal diameter unless otherwise described therein. Their alignment shall be tested in accordance with Sub-clause 509.9. The use of pipes and fittings other than those included in Table 5/2 shall be permitted provided that they hold a current Irish Agrément Board Roads and Bridges Certificate (or equivalent) stating that they are a suitable alternative to those listed in Table 5/2. Notwithstanding the above they shall comply with all other relevant requirements of the NRA Specification for Road Works.

2 Each duct shall be fitted with a pigmented, stranded polypropylene or approved equivalent rot-proof material draw rope of 5kN breaking load and having a design life of not less than 20 years, the ends of which shall be made fast to marker blocks as shown on the NRA Road Construction Details listed in Appendix 5/7. The ends of a duct shall be either sealed by removable stoppers immediately after it has been laid, or terminated in chambers of the type specified in Appendix 5/2.

3 Duct construction shall comply with the requirements of Appendix 5/2.

4 Ducts need not have watertight joints unless otherwise described in Appendix 5/2.
5 Joints in pipes for service ducts shall comply with the appropriate European Standard and with the following:

(i) Pipes for ducts shall be jointed so that no silt, grit, grout or concrete surround is able to enter the duct. Pipes with push-fit joints shall have a register to ensure that the pipe is fully pushed into the joint.

(ii) Joints in pipes to BS 3505 and IS EN 1452 shall comply with IS EN 1452-1 to 5 as appropriate.

6 The position of service ducts shall be marked when the trenches are backfilled and permanent marker blocks and location posts provided as described in Appendix 5/2.

7 Pipes for service ducts shall have nominal internal diameters of between 50mm and 150mm.

8 The colour of ducting pipes shall be in accordance with the UK National Joint Utilities Group publication “Guidelines on the Positioning and Colour Coding of Utilities’ Apparatus”. Any variation in the colour shall be described in Appendix 5/2.

Table 5/2: Pipes for Ducts

<table>
<thead>
<tr>
<th>Material</th>
<th>Standard</th>
<th>Particular Requirements</th>
</tr>
</thead>
<tbody>
<tr>
<td>Iron</td>
<td>IS EN 598 (Ductile iron)</td>
<td></td>
</tr>
<tr>
<td>Unplasticised polyvinyl-chloride (UPVC)</td>
<td>IS EN 1401 or BS 4660 or BS 5481 or BS 3506 (Class C) or IS EN 1452</td>
<td>When pipes to BS 3506 (Class C) are used, joints shall comply with BS 4346: Part 2</td>
</tr>
</tbody>
</table>

Pipes for use in motorway communications installations shall comply with the 1500 Series

525 Chambers for Ducts

1 Chambers for Ducts, other than those used in motorway communications installations, shall include manholes, catch-pits, inspection chambers, draw pits and walled soakaways. Chambers shall be of the type specified in Appendix 5/1, constructed in accordance with the NRA Road Construction Details listed in Appendix 5/7 as appropriate to that type. Inspection chambers not exceeding 1.3 metres in depth to invert, complete plastics units or other units in equivalent material surrounded by 150 mm of mix ST4 concrete may be used.

Chambers for use in motorway communications installations shall comply with the 1500 Series and Appendix 5/2. All ST concrete referred to in this Clause shall comply with the requirements of Concrete for Ancillary Purposes in the 2600 Series unless otherwise described in Appendix 5/1.

2 Foundations to chambers shall be of mix ST4 concrete.
3 Brickwork and Blockwork shall comply with the 2400 Series and be built with mortar designation (i) in English bond. The joints of brickwork where exposed shall be finished as specified for unpointed joints in the 2400 Series. The ends of all pipes shall be neatly built into the brickwork and finished flush with mortar designation (i).

4 Precast concrete chambers shall comply with BS 5911-3, IS EN 1917, IS 420 and the particular requirements described in Appendix 5/1. Cast insitu concrete chambers shall be constructed of mix ST4 concrete complying with Clause 2602 and the particular requirements described in Appendix 5/1.

5 Inverts and benchings shall have a screeded, ridged finish and shall have a smooth, high-strength concrete topping (minimum thickness 20mm) applied with a steel trowel before the concrete has set. The topping shall be produced, laid and finished in accordance with the relevant provisions of BS 8204: Part 2 and the following approximate mix proportions by weight shall be used: 1 part cement, 1 part natural sand and 2 parts single – sized coarse aggregate.

6 Where the depth of invert of chambers exceeds 900mm below the finished surface of the carriageway or the adjacent ground, manhole steps complying with IS EN 13101 shall be built in accordance with the NRA Road Construction Details listed in Appendix 5/7. Steelwork used for ladders, handholds and other fittings shall comply with IS EN 10084 and be galvanised in compliance with the requirements of the 1900 Series after fabrication. Threaded components shall also be galvanised in compliance with the requirements of the 1900 Series.

7 Excavation around chambers, except those described in Sub-clause 6 of this Clause, shall be backfilled with general fill material as described in Table 6/1 and compacted in compliance with the requirements of the 600 Series. Where mechanical compaction is impracticable, the excavation shall be backfilled with mix ST2 concrete. Where there are precast concrete access shafts to precast concrete chambers, the shafts shall be surrounded by a minimum thickness of 150mm of mix ST4 concrete, and the remaining excavation backfilled with general fill material as described in Table 6/1 compacted in compliance with Clause 612.

8 Chamber covers, gratings and frames shall be as described in Appendix 5/1 and shall comply with IS EN 124 and Sub-clauses 10 to 14 of this Clause.

(i) Products shall be designed and developed within an IS EN ISO 9001 system that has been assessed by a relevant recognised certification body. A relevant certification body shall have IS EN 45011 and IS EN ISO/IEC 17021 within its scope.

(ii) Products shall be type tested by a third party testing laboratory/organisation that has been accredited to IS EN ISO/IEC 17025 and has IS EN124 within its scope.

(iii) The certification process shall be overseen or audited by a single relevant accredited certification body meeting the requirements in (i) and (ii) above.

(iv) Copies of all certificates shall be forwarded to the Employers Representative along with audited product test reports.

(v) All documentation must be furnished in English or Irish.

(vi) Where gratings and frames are required to be locked the minimum size of the locking bolt shall be M16 and it shall be manufactured from stainless steel to prevent corrosion.

Amended as per Amendment No. 1, item 21.
9 Class D400 units shall incorporate a permanent non-rock feature either triangular point suspension, machined faces or cushioned or wedged seating.

10 Bolts supplied for loosely coupling separate sections of covers and gratings shall be steel hexagon headed, complying with the requirement of IS EN ISO 4016, 4018 and 4034 and be galvanised in compliance with the requirements of the 1900 Series. They shall not be less than M16 complete with hexagon nut and shall be provided with means to prevent undue tightening of unit sections.

11 Requirements for special duty covers for use in carriageways shall be as described in Appendix 5/1.

12 Two sets of lifting keys shall be delivered to the Employer's Representative for each type of cover supplied. At least two keyways shall be provided in each complete cover, one in each segment for segmental covers. A recess for a prising bar shall be incorporated in manhole covers unless other means of loosening the cover from the frame are provided.

13 Frames for chamber covers and gratings shall be set in cement mortar designation (i) complying with Clause 2404 or a proprietary quick setting mortar of equivalent strength.

Covers and gratings located within the carriageway, hardshoulder or hardstrip shall be set in a mortar with the following properties:

(i) The material shall be non-shrink;

(ii) The material shall have a minimum workable life of 15 minutes;

(iii) The compressive strength of the material shall exceed 30N/mm² in 3 hours;

(iv) The tensile strength of the material shall exceed 5N/mm² in 3 hours.

14 Where the adjustment or replacement of existing frames and covers or gratings is required, the units shall be taken up and refixed or replaced with new units complying with Sub-clauses 9 to 1423 of this Clause, or such other specification as required by the authority responsible for the utility to which the cover provides access. The frames and covers or gratings shall be laid on a mortar bed complying with Sub-clause 1424 of this Clause. The finished thickness of the mortar bed shall be between 10mm and 25mm. Unless otherwise described in Appendix 5/1 adjusted or replaced chamber frames and covers or gratings shall be set flush with the new surface. Any additional adjustments shall be by modifying the brickwork in compliance with Sub-clause 507.3 or by using a frame of a suitable depth.

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23 Amended as per Amendment No. 1, item 22.
24 Amended as per Amendment No. 1, item 23.
526  Ducts: Testing and Cleaning

1  Service ducts shall be checked by drawing a wooden mandrel, as shown on the NRA Road Construction Details listed in Appendix 5/7, through as the ducts are laid but where a set has to be given to the line of ducts the wooden mandrel shall be replaced by an iron mandrel 250mm long but of the same diameter as the wooden version.