

National Roads Authority

Manual of Contract Documents for Road Works (NRA MCDRW)

ERRATUM No. 1 (October 2009) to NRA Specification for Road Works Series 500 – Drainage & Service Ducts Dated January 2009

The NRA Specification for Road Works (NRA MCDRW), Series 500 – Drainage & Service Ducts, dated January 2009 is amended as follows:-

1. Page 5, Table 5/3
Replace “6/10” with “6/14” and replace “2/1” with “2/10”.
2. Page 5, Clause 503.3(v)
Delete “10% fines valuesoaked conditions” and replace with “resistance to fragmentation in Category LA₅₀ in accordance with IS EN 13242”.
3. Page 7, Clause 505.3 (i)
Delete “For type B and C”.
4. Page 8, Clauses 505.6 to 505.8
Delete **Type C** and replace with **Type A** in all cases (3 no.).
5. Page 9, Table 5/5
Delete Table 5/5 dated January 2009 and replace with Table 5/5 overleaf.
6. Page 11, Clause 507.10 (ii)
Delete “IS EN ISO/IEC 17021” and replace with “IS EN ISO/IEC 17025”
7. Page 12, Clause 507.13
Delete Sub-clause 507.13.
8. Page 12, Clause 507.17
Insert the following text at the end of the clause:

“Covers and gratings located within the carriageway, hardshoulder or hardstrip shall be set in a mortar with the following properties:

- (i) The material shall be non-shrink;
- (ii) The material shall have a minimum workable life of 15 minutes;
- (iii) The compressive strength of the material shall exceed 30N/mm² in 3 hours;
- (iv) The tensile strength of the material shall exceed 5N/mm² in 3 hours;”

9. Page 15, Clause 512
Delete “75 micron BS sieve” and replace with “63 micron sieve”.
10. Page 16, Clause 513.1
Delete “Unless otherwise described in Appendix 5/1”.
11. Page 17, Clause 514.6 (ii)
Delete “Table 5/7” and replace with “Table 5/8”.
12. Page 17, Table 5/7
Re-title table as “Table 5/8”.
13. Page 20, Clause 514.15 (vi)
Delete “Table 5/7” replace with “Table 5/8”.
14. Page 20, Clause 514.16 (a)
Delete “Table 5/7” and replace with “Table 5/8”.
15. Page 21, Clause 515.5
Delete “Table 5/8” and replace with “Table 5/9”.
16. Page 21, Table 5/8
Re-title table as “Table 5/9”.
17. This Erratum shall be implemented forthwith.
18. All technical enquiries or comments on this Erratum or the NRA Series 500 should be sent in writing to:

Specifications Section
National Roads Authority
St. Martins House
Waterloo Road
Dublin 4

Instructions For Use

The following version of the Manual of Contract Documents for Road Works (NRAMCDRW) incorporates the above changes.

Table 5/5: Grading and geometrical requirements for filter drain material.

| | Type A | Type B |
|--|--------------------------------------|--------------------------------------|
| Standard | IS EN 13242 | IS EN 13242 |
| Size, mm | 20/40 | 20/40 |
| Grading and oversize categories | G _c 80-20 | G _c 80-20 |
| Oversize category | - | - |
| Category for tolerances at mid-size sieves | G _{TNR} (no requirement) | G _{TNR} (no requirement) |
| Category for maximum fines | f _{NR} (no requirement) | f _{NR} (no requirement) |
| Category for Crushed or Broken Particles and Totally Rounded Particles | C _{90/3} | C _{NR} |
| Flakiness Index, FI | FI ₅₀ | FI _{NR} (no requirement) |
| Summary grading requirements | | |
| Sieve size, mm | Percentage by mass passing | |
| 80 | 100 | 100 |
| 63 | 98-100 | 98-100 |
| 40 | 80-99 | 80-99 |
| 20 | 0-20 | 0-20 |
| 10 | 0-5 | 0-5 |

DRAINAGE AND SERVICE DUCTS

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Drainage and Service Ducts

501 Pipes for Drainage and for Service Ducts

General

- 1 Where the term drain is used in this Series it shall be deemed to include the terms sewer and piped culvert up to 2000mm internal diameter. Box culverts shall be as described in Appendix 5/1.

The Contractor shall design the box or piped culverts listed in Appendix 1/10 in accordance with the requirements of the 100 Series for the Design of Permanent Works by the Contractor.

- 2 Drains, excluding those constructed using corrugated steel pipes exceeding 900 mm diameter, shall comply with this Series and any additional requirements described in Appendix 5/1. Drains constructed using corrugated steel pipes exceeding 900 mm diameter shall comply with the 2500 Series. Unless otherwise described in Appendix 5/1, only one type of pipe shall be used within any individual drain or service duct between consecutive chambers. The Contractor shall ensure that plastic pipes are not subject to deterioration due to sunlight during the period between manufacture and installation in the ground.

Pipes for Drainage

- 3 Pipes for drainage shall be selected from the alternatives in Table 5/1 and shall comply with the standards and particular requirements therein. The Contractor shall show that the pipes he selects have hydraulic flow capacity equal to that adopted in the hydraulic design of the system as described in Appendix 5/1. Pipes and fittings other than those included in Table 5/1 shall be permitted provided that they hold a current Irish Agrément Board Roads and Bridges Certificate (or equivalent) stating that they are a suitable alternative for the usage specified in Table 5/1.

On completion of the whole of the drainage works, the Contractor shall provide the Employer's Representative

with a schedule showing details of all pipe types used, including quality, joints and name of manufacturer.

Corrugated Steel Pipes Not Exceeding 900mm Diameter

- 4 Corrugated steel pipes shall be manufactured from either:
- (i) bolted segmental plate pipes complying with sub-Clause 6 of this Clause and having plate thicknesses as described in Appendix 5/1; or
 - (ii) galvanized steel sheet suitable for lock seam fabrication complying with IS EN 10327 grade DX51D + Z600, or aluminium coated steel sheet complying with AASHTO specification M274-87I (2004).

Corrugated steel pipes complying with (ii) above shall be manufactured from steel of minimum thickness 1.25 mm unless otherwise described in Appendix 5/1.

- 5 Where described in Appendix 5/1 corrugated steel pipes not exceeding 900 mm diameter shall be provided with additional protection of hot applied bitumen complying with AASHTO specification M190-95(2000), or an approved equivalent coating system.
- 6 Bolted segmental plate pipes shall meet the following requirements:
- (i) Steel for the plates shall comply with BS 1449: Part 1.1, Grade 3 or Grade 4, Condition HR.
 - (ii) After forming, the depth of the corrugations shall be within a tolerance of $\pm 6\%$ and the pitch of the corrugations within a tolerance of $\pm 4\%$ of the nominal dimensions. Plates shall have a minimum lip of 45 mm beyond each end crest. Cut edges shall be free from notches, gouges, rust or burrs.
 - (iii) Bolts and nuts for connecting plates shall comply with IS EN ISO 4014, IS EN 4017 and IS EN ISO 4032, for IS EN ISO 898-1 and IS EN 20898-2, property class 8.8, nominal size M20, or with BS 4395: Part 2, nominal size M20, or with IS EN ISO

- 898-1 and IS EN 20898-2, property class 10.9.
- (iv) When all the plates have been assembled, the nuts shall be tightened against a domed washer. The tightening shall be repeated if necessary to achieve the torque recommended by the manufacturer.
- (v) Steel plate shall be galvanized in compliance with the requirements of the 1900 Series. Plates shall be galvanized after forming the corrugations and completing all necessary cutting, punching and drilling.
- Units in which the zinc coating has been burned by welding or otherwise damaged in fabrication, transport or handling at Site shall be made good in compliance with the requirements of the 1900 Series. Bolts and nuts shall also be galvanized in compliance with the requirements of the 1900 Series.

Table 5/1: Pipes for Drainage

| Material | Usage | Standard | Particular Requirements |
|--|---|--|---|
| Vitrified Clay | Foul Drains | BS 65 or IS EN 295 | “Normal” pipes as defined in BS 65 |
| Concrete (With Portland Cement or sulphate-resisting cement when required in Appendix 5/1. Supersulphated cement shall not be used) | Foul & surface water drains not exceeding 900mm internal diameter | IS 6 or BS 5911-1 or IS EN 1916 | |
| | Surface water drains not exceeding 900mm diameter | IS 6 or IS 166 or BS 5911-110 | For use with joints complying with sub-Clause 504.4 |
| | Filter Drains | IS 6 or IS 166 or BS 5911-1 | Unperforated not exceeding 2m in length with open joints or castellated rebated joints with the total slot area between castellations being at least 100mm ² per metre length of pipe OR Perforated with circular holes not greater than 10mm or less than 3mm in diameter |
| | Drains exceeding 900mm diameter | IS 6 or IS 166 or BS 5911-1 and IS EN 1916 | Pipes having a concrete mix meeting the following requirements (Testing in accordance with the relevant BS and sampling in accordance with Clause 509): A total chloride ion content as described in Appendix 1/10 AND A 28-day minimum works cube strength of not less than 50 N/mm ² |
| Material | Usage | Standard | Particular Requirements |
| Iron | Foul & surface water drains | BS 437 (Cast Iron) | |
| | | IS EN 598 (Ductile Iron) | |
| Unplasticised polyvinyl-chloride (UPVC) | Foul & surface water drains | IS EN 1401 or BS 4660 or BS 5481 or IS EN 13476 or IS EN 13598-1 | |

| | | | |
|---|---|--|--|
| | Filter Drains | IS EN 1401 or BS 5481 | Perforated with not less than 100mm ² of holes per metre length of pipe. The perforations shall not reduce the pipe stiffness by more than 5%. Circular perforations not greater than 10mm or less than 3mm in diameter or rectangular slots not greater than 4mm or less than 0.6mm in width. |
| Plastics | Surface water drains | IS EN 13476 or Irish Agrément Board Roads and Bridges Certificate (or equivalent) | Unperforated with watertight joints and with a pipe stiffness and impact resistance as as described in Appendix 5/1 |
| | Filter drains | Irish Agrément Board Roads and Bridges Certificate (or equivalent) | Perforated and with a pipe stiffness and impact resistance as described in Appendix 5/1. |
| | Subsoil field drains | BS 4962 | |
| Material | Usage | Standard | Particular Requirements |
| Corrugated Steel | Surface Water drains, filter drains and piped culverts not exceeding 900mm diameter | AASHTO Specification M36 except as otherwise required in sub-Clauses 501.4,5 and 6 | |
| Drains exceeding 900mm diameter shall comply with the 2500 Series | | | |

Pipes for Service Ducts

7 Pipes for service ducts, excluding those in use in motorway communications installations, shall be selected from the alternatives in Table 5/2 and shall comply with the standards and particular requirements therein and also where applicable with the requirements of IS EN 50086-2-4 and IS EN 61386. Pipes for use in motorway communications installations shall comply with the 1500 series. Pipes for service ducts shall have a smooth internal bore without any sharp edges to the ends of pipes. They shall comply with any additional requirements described in Appendix 5/2, and be of 100 mm internal diameter unless otherwise described therein. Their alignment shall be tested in accordance with sub-Clause 509.9. The use of pipes and fittings other than those

included in Table 5/2 shall be permitted provided that they hold a current Irish Agrément Board Roads and Bridges Certificate (or equivalent) stating that they are a suitable alternative to those listed in Table 5/2. Notwithstanding the above they shall comply with all other relevant requirements of the NRA Specification for Road Works.

Table 5.2: Pipes for Ducts

| Material | Standard | Particular Requirements |
|---|---|--|
| Iron | IS EN 598 (Ductile iron) | |
| Unplasticised polyvinyl-chloride (UPVC) | IS EN 1401 or BS 4660 or BS 5481 or BS 3506 (Class C) or IS EN 1452 | When pipes to BS 3506 (Class C) are used, joints shall comply with BS 4346: Part 2 |

Pipes for use in motorway communications installations shall comply with the 1500 Series

8. Each duct shall be fitted with a pigmented, stranded polypropylene or approved equivalent rot-proof material draw rope of 5 kN breaking load and having a design life of not less than 20 years, the ends of which shall be made fast to marker blocks as shown on the NRA Road Construction Details listed in Appendix 5/7. The ends of a duct shall be either sealed by removable stoppers immediately after it has been laid, or terminated in chambers of the type specified in Appendix 5/2.

502 Excavation for Pipes and Chambers

1. Excavation shall comply with the General Requirements of the 600 Series and with the following:
 - (i) soft spots existing below the bottom of an excavation shall be removed and the resulting voids backfilled with Granular Material Type B complying with Clause 804 or pipe bedding material complying with Clause 503, both well compacted, or mix ST1 concrete;
 - (ii) any additional excavation below the bottom of an excavation that is required because the Contractor has allowed the bottom to become soft or otherwise unacceptable for the construction of the pipeline or

chambers shall be made good as described in sub-Clause 1(i) of this Clause;

- (iii) any excavation greater than the net volume required for the permanent Works below the level of any pipe surround shall be made good as described in (i) above.
- (iv) pipelines and chambers shall not be executed and completed in or beneath areas of embankment or fill until the execution and completion of the embankment has reached a level so as to ensure a minimum depth of cover of 1.2 metres above the top of the pipelines or, where pipes are specified in Appendix 5/1 with a cover of less than 1.2 metres, until the embankment has been formed and compacted to formation level under paved areas and to finished earthworks level in other areas.

503 Bedding, Laying and Surrounding of Pipes

1. Immediately following the excavation of the trench, the pipes shall be laid and jointed on the pipe bed. Pipes shall be laid so that each one is in contact with the bed throughout the length of its barrel. The pipes shall be laid at the level and gradients shown on the drawings and schedules in the contract. The deviation in level from that specified at any point shall not exceed 20 mm and in addition the algebraic difference of the deviation in level at any two points on each pipe shall not exceed 30 mm. In the case of socketed or sleeve jointed pipes the bed shall be cut away and removed at each socket or sleeve to give a clearance of at least 50 mm, or 100 mm for trenches in material designated as Hard Material, so that the socket or sleeve does not bear on the bed. Pipes shall be laid on setting blocks only where a concrete bed or cradle is used.

Pipes and fittings shall be examined for damage and the joint surfaces and components shall be cleaned immediately before laying. Measures shall be taken to prevent soil or other material from entering pipes, and to anchor each pipe to

prevent movement before the work is complete.

- 2 Pipes complying with BS 4962, which are corrugated coilable perforated pipes, shall be laid only by automatic single pass drain laying machines.
- 3 Drainage pipe and bedding combinations shall be selected from the alternatives described in Appendix 5/1. Pipe bedding, haunching and surrounding material shall be constructed as shown on the NRA Road Construction Details listed in Appendix 5/7, and shall comply with the following:
 - (i) For pipes on bed material shown on the NRA Road Construction Details listed in Appendix 5/7 the material shall comply with the following:
 - (a) granular material shall have a grading in accordance with Table 5/3;
 - (b) granular material shall have a water soluble sulphate content of less than 1.9 grams of sulphate (as SO₃) per litre when tested in accordance with IS EN 1744-1;
 - (c) concrete shall be mix ST4, backfilling shall not be carried out until after the concrete has cured.
 - (ii) For pipes on beds specified in Appendix 5/1 the granular material shall comply either with sub-Clause 3(i) of this Clause or with Table 5/4, with the water soluble sulphate content complying with (i) above.
 - (iii) Except for filter drains any Class 1 or Class 2 material above the bed, haunch and surround described above, shall be provided to a height of 300 mm above the top of the surround without any large stones or lumps of clay and in compliance with the 600 Series.
 - (iv) Unless otherwise described in Appendix 5/1 the materials used for the bedding, haunching and surrounding of filter drains shall comply with the appropriate bedding, haunching and surrounding materials specified in sub-Clauses 503.3.(i) to 503.3.(iv) and with the requirements for backfilling specified in sub-Clause 505.3.

Table 5/3: IS EN 13242, Coarse aggregate for pipe bedding, haunching and surrounding material

| IS EN 13242, Coarse aggregate | | |
|--|---|----------------------------|
| Category for general grading requirements | $G_c 80-20$ | |
| Category for tolerances at mid-size sieves | GT_{NR} (no requirements) | |
| Category for maximum values of fines content | Gravel- $f_{1.5}$ Crushed rock, recycled Aggregate – f_1 | |
| Nominal pipe diameter, mm | Aggregate size, mm | |
| | Graded | Single sized |
| Not exceeding 140 | - | 4/10 |
| Exceeding 140 but not exceeding 400 | 2/14 or 4/20 | 4/10, 6/14 or 10/20 |
| Exceeding 400 | 2/10, 4/20 or 4/40 | 4/10, 6/14, 10/20 or 20/40 |

Table 5/4: IS EN 13242, Fine and all-in aggregate for pipe bedding, haunching and surrounding material

| IS EN 13242, Fine and all-in aggregate | | |
|--|--|-----------------------------|
| | Fine | All-in |
| Category for general grading requirements | Category $G_F 80$ | Category $G_A 80$ |
| Category for tolerances on manufacturer's declared typical grading | GT_{FNR} (no requirement) | GT_{ANR} (no requirement) |
| Category for maximum values of fines content | Gravel – f_3 Crushed rock, recycled Aggregate – f_1 | |
| Nominal pipe diameter, mm | Aggregate size, mm | |
| | Fine | All-in |
| Not exceeding 140 | 0/1, 0/2, 0/4 Or 0/6 | 0/10 |
| Exceeding 140 but not exceeding 400 | | 0/10 or 0/20 |
| Exceeding 400 | | 0/10, 0/20 or 0/40 |

- (v) Granular materials used for bedding, haunching and surrounding of pipes shall have a resistance to fragmentation in Category LA₅₀ in accordance with IS EN 13242.

- 4 Except where the pipeline is to be tested in compliance with Clause 509 before backfilling, the completion of the bedding, haunching and surrounding of the pipes is to be carried out immediately after jointing. The bed, haunch and surround shall be brought up equally on both sides of the pipe ensuring that it is in contact with the underside of the pipe barrel and be carefully compacted in layers not exceeding 150 mm thickness ensuring full compaction next to the trench walls. Pipes shall be maintained to line and level during the bedding, haunching and surrounding operations. Where pipelines are to be tested before being covered, the bedding, haunching and surrounding material shall only be brought up sufficiently to support the pipeline and the joints shall be left exposed until the test is completed satisfactorily.
- 5 Duct construction shall comply with the requirements of Appendix 5/2.

504 Jointing of Pipes

- 1 Rigid joints shall mean joints made solid by caulking the sockets, or bolting together flanges integral with the pipes. Flexible joints shall mean joints made with deformable rings or gaskets held between pipe spigots and sockets, sleeves or collars.
- 2 Joints in surface water drains shall be watertight complying with sub-Clause 3 of this Clause or partly watertight complying with sub-Clause 4 of this Clause as described in Appendix 5/1. Foul drains shall have watertight joints. Filter drains shall have joints complying with sub-Clause 6 of this Clause. Ducts need not have watertight joints unless otherwise described in Appendix 5/2.
- 3 Watertight joints shall comply with the appropriate European Standards, the manufacturer's instructions and the following:
- (i) Rigid joints shall be used only where permitted in Appendix 5/1. Spigots and sockets of rigid joints may be caulked with tarred rope yarn or equivalent and the socket completely filled with mortar designation (i) complying with Clause 2404, excluding lime; a fillet of mortar being

- worked around the socket extending for a length of not less than 50 mm from the face of the socket. Iron pipes with open sockets shall have rigid joints caulked with lead wool or equivalent.
- (ii) Joints in UPVC pipes shall not be made with plastic solvent.
 - (iii) Flexible mechanical joints may be used with surface water pipes complying with BS 65.
 - (iv) Joints for cast iron pipes to BS 437 shall comply with IS EN 877.
 - (v) Joints in plastics pipes which have an Irish Agrément Board Roads and Bridges Certificate or British Board of Agrément Roads Certificate for Highway Drainage Products shall comply with the manufacturer's instructions.

- 4 Partly watertight joints for surface water drains shall be tested in accordance with sub-Clause 509.7 and shall be British Standard joints or non-British Standard joints. Push fit joints shall have a register to ensure that the pipe is fully pushed into the joint. Corrugated steel pipes of lock seam fabrication, not exceeding 900 mm diameter, shall be joined in accordance with the manufacturer's instructions. Bolted segmental plate pipe arches or circular pipes, not exceeding 900 mm diameter, shall be joined in accordance with sub-Clause 501.6 (iv) and the manufacturer's instructions.
- 5 Where a concrete bed, cradle, arch or surround is used with rigid pipes having flexible joints, joint filler board complying with Clause 1015 shall be placed in contact with the end of the socket at a pipe joint and shall extend through the full thickness of the concrete in contact with the pipe. Such joints in the concrete bed, haunch or surround shall be at intervals not exceeding 5 metres except where the spacing of joints in the pipe exceeds 5 metres when they shall be at each pipe joint.
- 6 Joints in pipes for filter drains shall comply with the appropriate European Standard and with the following:

- (i) Non-porous and unperforated concrete pipes with spigot and socket, rebated or ogee joints shall be laid with unsealed joints and with a gap of 10 mm between the end of the pipe and the inner end of the socket or rebate. The pipes shall be supported with tarred rope yarn or equivalent flexible jointing material within the sockets over the lower third of the circumference so that there are no vertical steps between one pipe and another. Such pipes shall only be used with Type B filter material as described in Clause 505.
 - (ii) The ends of perforated or castellated concrete pipes with rebated joints shall be pushed tightly together. The width of slots measured along the length of the pipeline formed by jointing castellated pipes shall not exceed 10 mm.
 - (iii) Perforated or slotted UPVC pipes with spigots and sockets or sleeves may be dry-jointed or jointed as described in sub-Clauses 3 and 4 of this Clause.
 - (iv) Other perforated pipes shall be jointed as unperforated pipes of the same material.
- 7** Joints in pipes for service ducts shall comply with the appropriate European Standard and with the following:
- (i) Pipes for ducts shall be jointed so that no silt, grit, grout or concrete surround is able to enter the duct. Pipes with push-fit joints shall have a register to ensure that the pipe is fully pushed into the joint.
 - (ii) Joints in pipes to BS 3505 & IS EN 1452 shall comply with IS EN 1452-1 to 5 as appropriate.
- be used within 300mm of any part of the pipe or joint.
- 2** Except where otherwise described in Appendix 5/1, trenches other than filter drain trenches shall be backfilled above the pipe surround material described in Clause 503 with Class 1 or 2 general fill material complying with the requirements of the 600 Series. Backfill of trenches, other than in carriageways and other paved areas, shall be brought up to ground level. Where the ground surface on the line of the trench consists of topsoil, the upper section of the backfill shall be topsoil of the thickness described, or of the same thickness and quality of soil as the surrounding ground. For trenches in carriageways or other paved areas the backfill shall be brought up to formation level, or sub-formation level where capping is required, unless a lower level is described in Appendix 5/1.
- 3** Trenches for Filter drains shall be backfilled as described in Appendix 5/1 and in accordance with the NRA Road Construction Details listed in Appendix 5/7. Filter materials for backfilling shall be selected from Table 5/5 and shall comply with the following:
- (i) geometrical requirements in accordance with Table 5/5 and IS EN 13242;
 - (ii) a resistance to fragmentation in Category LA50 in accordance with IS EN 13242;
 - (iii) a water-soluble sulphate content of less than 0.38% of sulphate (as SO₃) when tested in accordance with IS EN 1744-1, clause 10;
 - (iv) be non-plastic when tested in accordance with BS 1377: Part 2.

- 4** The filter material shall be deposited in layers each not exceeding 225 mm loose depth and each layer shall be lightly compacted.

Filter Drain Surface Stabilisation using a Geo-Synthetic Grid

- 5** A geo-synthetic grid is to be incorporated into the surface layer of filter drains as a means of reducing the risk of stone scatter. The geo-synthetic grid locks the aggregate particles together forming a stable surface

505 Backfilling of Trenches and Filter Drains

- 1** Backfilling shall be undertaken immediately after the specified operations preceding it have been completed. The material as described in the Contract shall be deposited in layers, compacted in accordance with requirements of the 600 Series. Care should be taken to compact the material evenly without dislodging or damaging pipes. Power rammers are not to

that enables fully loaded heavy goods vehicles (HGVs) to be driven into and out of the drain, reducing the risk of the wheels sinking into the drain and the vehicle becoming stranded.

- 6 The geo-synthetic grid shall be high density polyethylene or polypropylene, have a nominal ultimate tensile strength in each direction of 30kNm and have a minimum aperture of each grid similar to the maximum Type A aggregate size, around 60mm.
- 7 The Type B backfill shall be stopped approximately 150mm to 200mm below the top of the filter drain. The Type B material shall be lightly compacted in accordance with sub-clauses 1 and 4 of this clause. Care should be taken not to crush the material as this could result in small pieces of aggregate and a consequent reduction in the effectiveness of the reinforcement. A layer of Type A material should be spread over the compacted surface to provide interlock and the geo-synthetic grid placed over this.
- 8 The geo-synthetic grid shall be placed flat over the full width of the drain and shall be between 100mm and 150mm below the surface of the filter drain. The drain shall be brought up to surface level with Type A material as shown on the NRA Road Construction Details listed in Appendix 5/7.
- 9 The minimum overlap at joints in the geo-synthetic grid shall be 300mm.
- 10 Following filling, the final surface of the drain shall be compacted to lock the aggregate and reinforcement.

Service Ducts

- 11 The position of service ducts shall be marked when the trenches are backfilled and permanent marker blocks and location posts provided as described in Appendix 5/2.

Table 5/5: Grading and geometrical requirements for filter drain material.

| | Type A | Type B |
|--|--------------------------------------|--------------------------------------|
| Standard | IS EN 13242 | IS EN 13242 |
| Size, mm | 20/40 | 20/40 |
| Grading and oversize categories | G _C 80-20 | G _C 80-20 |
| Oversize category | - | - |
| Category for tolerances at mid-size sieves | G _{TNR} (no requirement) | G _{TNR} (no requirement) |
| Category for maximum fines | f _{NR} (no requirement) | f _{NR} (no requirement) |
| Category for Crushed or Broken Particles and Totally Rounded Particles | C _{90/3} | C _{NR} |
| Flakiness Index, FI | FI ₅₀ | FI _{NR} (no requirement) |
| Summary grading requirements | | |
| Sieve size, mm | Percentage by mass passing | |
| 80 | 100 | 100 |
| 63 | 98-100 | 98-100 |
| 40 | 80-99 | 80-99 |
| 20 | 0-20 | 0-20 |
| 10 | 0-5 | 0-5 |

506 Connecting to Existing Sewers, Drains, Chambers and Channels

- 1 Where described in Appendix 5/1, existing sewers and drains shall be extended, connected and jointed to new drains, chambers or channels. All such connections shall be made during the construction of the new sewer, drain or other work and their positions recorded by the Contractor who shall hand to the Employer's Representative a copy of the record of the connections made the previous day. Where pipe connections are made to existing brick concrete or stone sewers, drains, chambers or channels, the pipes shall be well and tightly built into the concrete, brick or masonry work and be so placed as to discharge at an angle not greater than 60° to the direction of flow of the sewer, drain or channel and with the end of the pipe carefully cut to the necessary angle. Where the connections are between pipe sewers or drains, special connecting pipes shall be laid and jointed as described in Appendix 5/1.
- 2 Before entering or breaking into an existing sewer or drain, the Contractor shall give notice of his intention to do so to the authority responsible for the pipeline to which the connection is to be made.
- 3 Existing sewers or drains no longer required shall, as required by Appendix 5/1, be:
 - (i) sealed with mix ST2 concrete,
 - (ii) removed and replaced with acceptable fill material deposited in layers and compacted in accordance with the requirements of the 600 Series or
 - (iii) grouted with a 1:10, cement: PFA mix. The grout shall use the minimum quantity of water to ensure the fluidity necessary to render it capable of being pumped to the ends of the pipe. It shall be used within one hour of mixing but when the mix contains a retarding admixture this time may be extended in accordance with the manufacturer's instructions. The cement shall be CEM I Portland cement complying with IS EN 197 : Part 1 and the pulverised-fuel ash (PFA) with BS 3892: Part 2, fineness

to Zone B and sulphate content not exceeding 1.5%.

- 4 All existing foul and surface water drainage shall be maintained until the permanent drainage for the execution and completion of the Works shall have been executed, completed and functioning satisfactorily including its discharge to approved outfalls
- 5 Ground profiles shall at all times be maintained to shed surface water efficiently and directly into the nearest drain and to prevent penetration of water into or below existing pavements
- 6 Existing drainage retained in the execution and completion of the Works shall be flushed and cleared of debris and all repairs carried out to ensure drains shall fulfil the requirements of the Contract. The clearing and repair of any existing drainage shall not have any negative impact on local watercourses.

507 Chambers

- 1 Chambers, other than those used in motorway communications installations, shall include manholes, catchpits, inspection chambers, draw pits and walled soakaways. Chambers shall be of the type specified in Appendix 5/1, constructed in accordance with the NRA Road Construction Details listed in Appendix 5/7 as appropriate to that type. Chambers for use in motorway communications installations shall comply with the 1500 Series and Appendix 5/2. All ST concrete referred to in this Clause shall comply with the requirements of Concrete for Ancillary Purposes in the 2600 Series unless otherwise described in Appendix 5/1.
- 2 Foundations to chambers shall be of mix ST4 concrete. Channels for chambers shall be formed and finished smooth in the foundation concrete or constructed of preformed half circle channels, with sides benched in mix ST4 concrete, or mortar designation (i) complying with the requirements of the 2400 Series excluding lime. Alternatively for inspection chambers not exceeding 1.3 metres in depth to invert, complete plastics units or other units in

- equivalent material surrounded by 150 mm of mix ST4 concrete may be used.
- 3** Brickwork and Blockwork shall comply with the 2400 Series and be built with mortar designation (i) in English bond. The joints of brickwork where exposed shall be finished as specified for unpointed joints in the 2400 Series. The ends of all pipes shall be neatly built into the brickwork and finished flush with mortar designation (i).
- 4** Precast concrete chambers shall comply with BS 5911-3, IS EN 1917, IS 420 and the particular requirements described in Appendix 5/1. Cast in situ concrete chambers shall be constructed of mix ST4 concrete complying with Clause 2602 and the particular requirements described in Appendix 5/1.
- 5** Inverts and benchings shall have a screeded, ridged finish and shall have a smooth, high - strength concrete topping (minimum thickness 20mm) applied with a steel trowel before the concrete has set. The topping shall be produced, laid and finished in accordance with the relevant provisions of BS 8204: Part 2 and the following approximate mix proportions by weight shall be used: 1 part cement, 1 part natural sand and 2 parts single – sized coarse aggregate.
- 6** Corrugated galvanized steel chambers shall comply with Clause 501 with in situ mix ST4 concrete inverts and precast concrete cover slabs complying with BS 5911-3 and IS EN 1917 and the particular requirements described in Appendix 5/1. They shall be surrounded with well graded granular material Class 6M as described in Table 6/1 compacted in accordance with the requirements of the 600 Series.
- 7** Where the depth of invert of chambers exceeds 900 mm below the finished surface of the carriageway or the adjacent ground, manhole steps complying with IS EN 13101 shall be built in accordance with the NRA Road Construction Details listed in Appendix 5/7. Steelwork used for ladders, handholds and other fittings shall comply with IS EN 10084 and be galvanized in compliance with the requirements of the 1900 Series after fabrication. Threaded components shall also be galvanized in compliance with the requirements of the 1900 Series.
- 8** Excavation around chambers, except those described in sub-Clause 6 of this Clause, shall be backfilled with general fill material as described in Table 6/1 and compacted in compliance with the requirements of the 600 Series. Where mechanical compaction is impracticable, the excavation shall be backfilled with mix ST2 concrete. Where there are precast concrete access shafts to precast concrete chambers, the shafts shall be surrounded by a minimum thickness of 150 mm of mix ST4 concrete, and the remaining excavation backfilled with general fill material as described in Table 6/1 compacted in compliance with Clause 612.
- 9** Chambers for foul sewers and drains shall be tested for watertightness as and where required in Appendix 5/1.
- 10** Chamber covers, gratings and frames shall be as described in Appendix 5/1 and shall comply with IS EN 124 and sub-Clauses 10 to 16 of this Clause.
- (i) Products shall be designed and developed within an IS EN ISO 9001 system that has been assessed by a relevant recognised certification body. A relevant certification body shall have IS EN 45011 and IS EN ISO/IEC 17021 within its scope.
 - (ii) Products shall be type tested by a third party testing laboratory/organisation that has been accredited to IS EN ISO/IEC 17025 and has IS EN124 within its scope.
 - (iii) The certification process shall be overseen or audited by a single relevant accredited certification body meeting the requirements in (i) and (ii) above.
 - (iv) Copies of all certificates shall be forwarded to the Employers Representative along with audited product test reports.
 - (v) All documentation must be furnished in English or Irish.
 - (vi) Where gratings and frames are required to be locked the minimum size of the locking bolt shall be M16 and it shall be manufactured from stainless steel to prevent corrosion.

- 11 Class D400 units shall incorporate a permanent non-rock feature either triangular point suspension, machined faces or cushioned or wedged seating.
- 12 Bolts supplied for loosely coupling separate sections of covers and gratings shall be steel hexagon headed, complying with the requirement of IS EN ISO 4016, 4018 and 4034 and be galvanised in compliance with the requirements of the 1900 Series. They shall not be less than M16 complete with hexagon nut and shall be provided with means to prevent undue tightening of unit sections.
- 13 Not Used.
- 14 Requirements for special duty covers for use in carriageways shall be as described in Appendix 5/1.
- 15 Gratings for catchpit chambers shall have a minimum waterway area as described in Appendix 5/1.
- 16 Two sets of lifting keys shall be delivered to the Employer's Representative for each type of cover supplied. At least two keyways shall be provided in each complete cover, one in each segment for segmental covers. A recess for a prising bar shall be incorporated in manhole covers unless other means of loosening the cover from the frame are provided.
- 17 Frames for chamber covers and gratings shall be set in cement mortar designation (i) complying with Clause 2404 or a proprietary quick setting mortar of equivalent strength.

Covers and gratings located within the carriageway, hardshoulder or hardstrip shall be set in a mortar with the following properties:

- (i) The material shall be non-shrink;
- (ii) The material shall have a minimum workable life of 15 minutes;
- (iii) The compressive strength of the material shall exceed 30N/mm² in 3 hours;
- (iv) The tensile strength of the material shall exceed 5N/mm² in 3 hours;

- 18 For all pipelines except those constructed with corrugated pipes the nearest joint to any chamber shall be not more than 500 mm from the inner face of the wall and shall not be restricted by any concrete. Between this and the next joint, the length of the articulated pipe shall be in accordance with Table 5/6.

Table 5/6: Length of Articulated Pipe.

| Nominal Pipe Diameter (mm) | Length of Pipe (mm) |
|----------------------------|---------------------|
| 450 and less | 500 to 750 |
| Greater than 450 | 750 to 1000 |

- 19 Where the adjustment or replacement of existing frames and covers or gratings is required, the units shall be taken up and refixed or replaced with new units complying with sub-Clauses 9 to 15 of this Clause, or such other specification as required by the authority responsible for the utility to which the cover provides access. The frames and covers or gratings shall be laid on a mortar bed complying with sub-Clause 17 of this Clause. The finished thickness of the mortar bed shall be between 10 mm and 25 mm. Unless otherwise described in Appendix 5/1 adjusted or replaced chamber frames and covers or gratings shall be set flush with the new surface. Any additional adjustments shall be by modifying the brickwork in compliance with sub-Clause 507.3 or by using a frame of a suitable depth.

508 Gullies and Pipe Junctions

- 1 Gullies shall be untrapped, unless otherwise described in Appendix 5/1, and be in accordance with the NRA Road Construction Details listed in Appendix 5/7.
- 2 Gullies shall be constructed so that no part of the spout or trap has a cross-sectional area less than 2/3rd that of the outlet. The depth of water seal in trapped gullies shall be not less than 50 mm.
- 3 Precast concrete gullies shall comply with BS 5911-6. In situ concrete gullies shall be constructed of mix ST4 concrete or concrete blockwork of 225mm minimum thickness and be as described in Appendix 5/1.

Where in situ concrete gullies are formed with permanent shuttering, such shuttering shall have a current Irish Agrément Board Roads and Bridges Certificate or equivalent.

- 4 Cast iron and cast steel gully gratings shall comply with IS EN 124, sub-Clauses 10 to 16 of Clause 507.7 and shall be in accordance with the NRA Road Construction Details listed in Appendix 5/7. They shall be of the classes and sizes described in Appendix 5/1. All gully gratings shall be fitted with a durable, non-rust locking device.
- 5 The upper surface of gully gratings shall be flat unless otherwise described in Appendix 5/1. Slots in gratings or between gratings and frames shall not be orientated parallel to the direction of traffic. Frames shall be bedded on mortar complying with sub-Clause 507.17. Brickwork shall comply with sub-Clause 507.3.
- 6 Backfilling to precast gullies shall be carried out up to sub-formation level with general fill material Class 1, as described in Table 6/1 compacted in compliance with the requirements of the 600 Series. Where mechanical compaction is impracticable, the backfilling shall be in mix ST2 concrete. The remainder of the backfilling shall be in appropriate capping and road pavement materials except that where mechanical compaction of capping or granular sub-base is impracticable mix ST2 concrete shall be used.
- 7 Gully connection pipes shall be either flexible or rigid not exceeding 0.7 m in length with flexible joints for a distance of 2 m from the gully and shall be in accordance with sub-Clause 507.18 when entering chambers. Junction pipes shall be manufactured of the same type and class of material as the remainder of the pipes in the run. Junction pipes which are laid but not immediately connected, shall be fitted with temporary stoppers or seals and the position of all such junctions shall be clearly defined by means of stakes or tracing wires properly marked or labelled. Saddles may be used to form junctions only where permitted in Appendix 5/1. No internal projections greater than 5 mm will be permitted. Saddles for plastics pipes shall be installed in accordance with the

manufacturer's recommendations. Saddles with clay pipes shall be jointed with mortar designation (i) complying with Clause 2404, excluding lime. Saddles and pipes shall be surrounded with mix ST2 concrete.

- 8 Where the adjustment or replacement of existing frames and gratings or kerb type gully covers is required, the units shall be taken up and refixed or replaced with new units complying with sub-Clauses 4 and 5 of this Clause or such other specification required by the Road Authority at a level, unless otherwise described in Appendix 5/1, 6 mm below the adjoining road surface on a mortar bed complying with sub-Clause 507.17. The thickness of the mortar bed shall be between 10 mm and 25 mm. Any additional adjustment shall be made by modifying the brickwork in compliance with sub-Clause 507.3 or by using a frame of suitable depth.

509 Testing and Cleaning

- 1 Drains required in Appendix 5/1 to have watertight joints shall be tested as described in Appendix 1/5 in sections, eg. between chambers, by means of the air test described in sub-Clause 2 of this Clause. If a pipeline is rejected because of a failed air test, as part of the rectification work, a water test as described in sub-Clause 3 of this Clause may be carried out as an alternative acceptability test. Before testing, the ends of the pipeline to be tested, including those of short branches, shall be plugged and sealed.
- 2 For the pipeline air test, air shall be pumped in by suitable means until a stable pressure of 100 mm head of water is indicated in a U-tube connected to the system. The air pressure shall not fall to less than 75 mm head of water during a period of 5 minutes without further pumping, after an initial period to allow stabilization. Drains with traps shall be tested to 50 mm head of water and the permissible loss shall then be no more than 13 mm head of water in 5 minutes without further pumping after the initial stabilising period.
- 3 For the pipeline water test, the pipes shall be filled with water under a head of not

less than 1.2 m above the crown of the pipe at the high end and not more than 6 m above the pipe at the low end. Steeply graded pipelines shall be tested in sections so that the above maximum is not exceeded. The test shall commence two hours after filling the test section at which time the level of water at the vertical feed pipe shall be made up to produce the required 1.2 m minimum test head. The loss of water over a 30 minute period shall be measured by adding water at regular 10 minute intervals to restore the original water level and recording the amounts so added. The drain will have passed the test if the volume of water added does not exceed one litre per hour per linear metre of drain per metre of nominal internal diameter.

- 4 All pipelines less than 350 mm diameter, excluding service ducts shall be checked by drawing through each completed length of pipe a spherical mandrel of a diameter 10 mm less than the smallest internal pipe diameter permitted by the tolerances specified for the pipes being tested.
- 5 During the progress of the Works all existing chambers, gullies and rodding eyes shall be kept clean and free from obstruction. On completion of the whole of the Works, all chambers, gullies and drains including verge/surface water drains and filter drains but excluding all fin and narrow filter drains shall be flushed from end to end with water and left free from obstructions. Unless otherwise required in Appendix 5/9 all carrier, foul and filter drains shall be surveyed by Closed Circuit Television (CCTV) in accordance with the relevant requirements of the 2900 Series. Catchpit chambers shall be left clean and free from silt.
- 6 The pipes and filter material of filter drains shall at all times be left clean and free from silt and obstruction.
- 7 The Contractor shall carry out trial holes upon completion of filter drain trenches as directed by the Employer's Representative to demonstrate that the filter material has not become clogged with other material during construction. If the filter material is clogged then the contractor shall replace it with clean material.

- 8 Where described in Appendix 1/5, samples of one or more partly watertight joints for pipelines up to and including 900 mm diameter shall be tested with a head of water kept level with the crown of the pipe. The joint will not be accepted if the flow through the joint in litres per minute exceeds 20 times the square of the nominal internal diameter of the pipe in metres.
- 9 Permeability tests shall be as described in Appendix 5/1.
- 10 For concrete pipes exceeding 900 mm internal diameter the Contractor shall obtain and make available test certificates provided by the manufacturer, based on the following sampling rates:
 - (i) For the hydraulic or hydrostatic test and for the works proof load test, samples shall be taken at random from each batch of consecutively manufactured pipes of each diameter, joint type and strength class as described in Table 5/7.
 - (ii) For the maximum load test, one pipe shall be taken at random from those have been selected for, and have passed the proof load test. If this pipe fails, a further two pipes taken from those which have been selected for, and have passed the proof load test shall be tested and if either pipe then fails, the whole batch shall be rejected.
- 11 Service ducts shall be checked by drawing a wooden mandrel, as shown on the NRA Road Construction Details listed in Appendix 5/7, through as the ducts are laid but where a set has to be given to the line of ducts the wooden mandrel shall be replaced by an iron mandrel 250 mm long but of the same diameter as the wooden version.

510 Surface Water Channels and Drainage Channel Blocks

- 1 Surface water channels and drainage channel blocks shall be constructed as described in Appendix 5/3.

- 2 Surface water channels shall comply with the requirements of the 1100 Series for In Situ Channels.
- 3 Drainage channel blocks shall comply with the requirements of the 1100 Series for Precast Channels.

511 Land Drains

- 1 Existing land drains which are permanently severed by the Works shall be located and connected into a new drain, pipe or ditch all as described in Appendix 5/1. The lengths remaining within the Works shall be cleaned out from the new drain trench face as necessary. Any pipe disturbed by the Works shall be re-laid to ensure a free discharge into the new drain. Disused ends of intercepted land drains be adequately sealed with mix ST2 concrete in compliance with the requirements of the 2600 Series for Concrete for Ancillary Purposes.
- 2 Where an existing land drain is exposed and severed by temporary trench excavation, the Contractor shall mark the position of the drain and record it. The drain shall be diverted into an existing drain or watercourse. Alternatively, the normal functioning of the drain shall be continued by the construction of a pipeline or channel adequately supported across the excavation, until permanent restoration is made on the original line.
- 3 The Contractor shall notify the Employer's Representative of any land drain which is blocked or is otherwise defective when the drain is first exposed.

Severed mole drains shall be led straight into new drains; alternatively they shall where required in Appendix 5/1 be intercepted by the construction of a land drain. Where they have been disturbed mole channels shall be cleaned out and filled locally with Type B filter material or as otherwise described in Appendix 5/1.

512 Backfilling to Pipe Bays and Verges on Bridges

- 1 Unless otherwise described in Appendix 5/1, filling to pipe bays and verges on bridges shall be well graded granular material not exceeding 20 mm size containing not more than 3% of material passing the 63 micron sieve and with a uniformity coefficient of more than 5. It shall be laid and compacted in compliance with Clause 505. The material shall meet the sulphate requirement described in sub-Clause 503.3.

Table 5/7: Sampling Rates

| No of Pipes In Batch | Sample Size | If All Pass | If 2 or more Fail | If 1 Fails take a Further Sample of | If 1 or more of the further Sample Fails |
|----------------------|--------------------|-------------|---|-------------------------------------|---|
| <40 | 2 | ACCEPT | REJECT or test ALL remaining pipes in batch, failing pipes to be rejected | 2 | REJECT or test ALL remaining pipes in batch, failing pipes to be rejected |
| 41-60 | 3 | | | 3 | |
| 61-80 | 4 | | | 4 | |
| 80-100 | 5 | | | 5 | |
| 101+ | 5 per batch of 100 | | | 5 | |

513 Permeable Backing to Earth Retaining Structures

1 Permeable backing shall consist of one of the following materials:

- (i) A minimum thickness of 300 mm of granular material of 20mm nominal size, satisfying the following criteria:

Piping ratio, defined as

$$\frac{15 \text{ per cent size of the drainage material}}{85 \text{ per cent size of the backfill material}}, < 5$$

Permeability ratio, defined as

$$\frac{15 \text{ per cent size of the drainage material}}{15 \text{ per cent size of the backfill material}}, < 5$$

where the per cent size of a material is the size of particle corresponding to the given per cent ordinate of the particle size distribution graph.

- (ii) Porous no-fines concrete, cast in situ 225 mm thick complying with the requirements of the requirements of the 2600 Series.
- (iii) Precast hollow concrete blocks complying with IS EN 772-2 & 3 laid in stretcher bond with dry joints in 225 mm thick walling with holes vertical.

514 Fin Drains

General

1 Fin drains shall comply with this Clause and the special requirements described in Appendix 5/4.

The terms thickness, width, height and core shall have the meanings indicated on NRA Road Construction Details listed in Appendix 5/7 unless otherwise described in Appendix 5/4.

Drain Types 5 & 6 shall be as show on the NRA Road Construction Details listed in Appendix 5/7

The term fin drain shall mean a planar geocomposite structure designed to perform the same function as a narrow filter drain.

2 Where fin drains are designed for lateral entry of water from one side only the requirements for flow rates in sub-Clauses 4 and 5 of this Clause shall apply to the face or plane designed to admit or transmit water.

3 The materials of which the drain is made shall be treated so that they are protected from the deleterious effects of short term exposure to ultraviolet light, and shall be resistant to degradation by acids, alkalis, common chemicals, bacteria, fungi and moulds occurring in soils and road construction materials. After exposure to ultraviolet light the Employer's Representative may require evidence that the materials still comply with the requirements of this Clause. The drain shall be protected

from damage and ultraviolet light and be labelled to identify the grade and manufacturer or supplier.

- 4 Where necessary, the side intended for entry of water and the direction of in-plane flow shall be identified.

Geotextile

- 5 The geotextile shall:
- (i) in both machine and cross-machine directions, sustain a tensile load of not less than 5.0 N/m at break and have a minimum failure strain of 10% when determined in accordance with IS EN ISO 10319;
 - (ii) have a minimum puncture resistance of 1200 N when determined in accordance with IS EN ISO 12236;
 - (iii) have a minimum tear resistance of 200 N when determined in accordance with ASTM Standard D4533-91 (1996);
 - (iv) have a size distribution of pore openings such that the apparent opening size O_{90} when determined in accordance with IS EN ISO 12956, or other appropriate test, is as stated in Appendix 5/4;
 - (v) allow water to flow through it, in either direction, normal to its principal plane at a rate of not less than that stated in Appendix 5/4 under a constant head of water of 100 mm and a maximum breakthrough head of 50 mm when determined in accordance with IS EN ISO 12958.

Composite Drain

- 6 The composite drain shall:
- (i) have a flow rate through each face of the drain of more than 75% of the value specified in sub-Clause 4(v) of this Clause on the side or sides where inflow occurs. This value may be found by either:
 - (a) direct measurement of the composite drain using IS EN ISO 12958; or
 - (b) calculation based on the flow rate obtained by the standard test in IS EN ISO 12958 and the percentage contact area of the

drainage core obtained in accordance with sub-Clause 13 of this Clause;

- (ii) have values of long term in-plane flow rates as stated in Appendix 5/4 when determined in accordance with sub-Clauses 14 and 15 of this Clause. The values of hydraulic gradient and minimum applied stresses shall be as given in Table 5/8.

TABLE 5/8: Applied Stresses (kN/m²) and Hydraulic Gradient

| RCD/500/31 Drain Type | Drain Type 5 | Drain Type 6 | Drain Type 7 |
|---|-----------------|-----------------|-----------------|
| Sub-Clause 14: Normal Stress | 50 | 50 | 50 |
| Shear Stress | 10 | 10 | 10 |
| Sub-Clause 15: Normal Stress | 100 | 100 | 100 |
| Sub-Clause 15: Hydraulic Gradient | 0.1 & 1.0 | 1.0 | 1.0 |

Joints

- 7 Fin drains shall be capable of being jointed longitudinally or laterally into pipe systems or chambers for inflow and outflow purposes and be self jointing either directly or through purpose made attachment pieces for forming continuous drain lengths. All such joints shall be formed so as to prevent the ingress of soil particles or other extraneous material into the drain.

Fin drain joints transverse to the direction of flow shall have values of in-plane flow rates not less than that required by sub-Clause 5(ii) of this Clause.

Fin drain joints parallel to the direction of flow and any exposed edges shall be protected from the ingress of soil by a geotextile wrapping with a minimum overlap of 150 mm.

Pipes

- 8 For drain Type 6, in accordance with the NRA Road Construction Details listed in Appendix 5/7, pipes shall be perforated or porous and comply with sub-Clause 501.3. Pipes

complying with BS 4962: 1989 shall have a minimum Ultimate Pipe Stiffness (STES) value of 1400 N/m².

For drain Type 7, in accordance with the NRA Road Construction Details listed in Appendix 5/7, pipes shall be unperforated thermoplastics pipe complying with IS EN 1401, IS EN 1852-1 or IS EN 12666-1 slotted longitudinally along the top surface and stress relief treated (if required) in accordance with the system manufacturer's published specification.

- 9 Pipe joints shall comply with the requirements of the relevant European Irish or British Standard for the pipe used or Clause 518.

Backfill and Surround Material

- 10 Pipe surround material for drain Types 6 and 7, in accordance with the NRA Road Construction Details listed in Appendix 5/7, shall comply with sub-Clause 503.3(i) or 503.3(ii) or Type B material complying with sub-Clause 505.3.

Where fin drains are installed in a trench, backfill material shall be the original as-dug material from the trench unless otherwise specified in Appendix 5/4.

Dimensions

- 11 Unless otherwise described in Appendix 5/4 the dimensions of the fin drain shall be as shown on the NRA Road Construction Details listed in Appendix 5/7. The pipe diameter shall be as stated in Appendix 5/4. The drain slope angle (α) as shown on the NRA Road Construction Details listed in Appendix 5/7 shall be not greater than 15% from the vertical unless otherwise stated in Appendix 5/4.

Installation and Handling

- 12 Installation of fin drains shall be as shown on the NRA Road Construction Details listed in Appendix 5/7. Where fin drains are assembled on site the assembly area shall be clean and dry and free of wind-borne pollutants. Any material which becomes contaminated must be replaced. No geotextile or core material shall be exposed to daylight (or any source of ultraviolet radiation) for a

period exceeding a cumulative total of 50 hours. Any geotextile or core material exposed to daylight (or any source of ultraviolet light) for a period exceeding a cumulative total of 50 hours shall be replaced unless it can be demonstrated that the materials of the drain still comply with the requirements of this Clause.

Where fin drains are laid in trench, the trench bottom shall be free of irregularities and to the required levels given in Appendix 5/4. Rock and other hard protuberances shall be removed and any excess cut in the trench bottom filled and compacted back to the required grade with suitable excavated or imported material.

The drain shall be laid with the appropriate face against the side of the trench adjacent to the carriageway and in the appropriate direction. This side of the trench shall have walls sufficiently clean to enable the fin drain to come into close contact with the wall when the trench is backfilled and compacted. Compaction shall be in accordance with Clause 612. Fin drains installed as part of the permanent Works shall be protected from surface water, contamination, and accidental damage during construction.

The fin drain, pipe surround and backfill shall be installed so as to cause no damage to the fin drain. Where any damage does occur, the damaged materials shall be replaced by new material.

After the installation of the fin drain has been completed a marker tape shall be laid approximately 75 mm above the fin drain in the position shown on the NRA Road Construction Details listed in Appendix 5/7. The tapes shall be green self-coloured PVC or polythene plastic not less than 0.1 mm thick and 150 mm wide.

Identification

- 13 The Contractor shall obtain and make available the following information for each separate consignment of fin drain delivered to Site:
- (i) geotextile and core name, grade/number and mass per unit area;
 - (ii) names and addresses of system producer, and geotextile, core and pipe manufacturers;

- (iii) manufacturing characteristics and constituents of geotextile and core. This shall include composition and type of constituent filaments, threads, fibres, films, tapes and other components;
- (iv) consignment number and delivery date;
- (v) a copy of the site delivery note.

Test Method for the Percentage Contact Area of Drainage Core

- 14**
- (i) The test determines the area of one face of a drainage core which will be in contact with a geotextile filter as a percentage.
 - (ii) The apparatus required is as follows:
 - (a) loading device able to apply a compressive load of at least 2 kN and having a flat steel base;
 - (b) flat steel loading plate 200 mm x 200 mm;
 - (c) printers ink and roller (or pad);
 - (d) sheet of thin compressible rubber;
 - (e) planimeter.
 - (iii) The test procedure shall be as follows. Cut three representative test specimens 200 mm x 200 mm (± 2 mm). Apply ink to one 200 mm x 200 mm face of a specimen and cover with a sheet of plain paper and a thin compressible rubber sheet. Place the prepared specimen in to the loading device and gradually apply the load of 2 kN and maintain for 5 minutes. Release the load and remove the specimen and separate it from the paper. Using the planimeter find the total area of the paper which has received an imprint. Repeat for all specimens.
 - (iv) The percentage contact area = $\frac{\text{total area of imprint}}{\text{area of test specimen}} \times 100$
 - (v) The report shall include:
 - (a) reference to this method;
 - (b) sample identification details;

- (c) individual and mean percentage contact areas;
- (d) details of any deviation from the specified test procedure.
- (vi) Alternative methods of determining the percentage contact area may be employed with the prior approval of the Employer's Representative.

Test Method for Determining the Thickness of Fin Drains Under Specified Normal and Shear Stresses

- 15**
- (i) The test determines the thickness of the fin drain under sustained normal and shear stresses. A long term thickness (at 100,000 hours) is calculated by extrapolation and a short term equivalent normal load which produces the long term thickness is determined.
 - (ii) The apparatus required is as follows:
 - (a) a suitable compression testing machine, which shall have a vertical travel at least the nominal thickness of the specimen. It shall be capable of sustaining the necessary loads to within 1% accuracy for the duration of the test;
 - (b) the compression testing apparatus, which shall include a fixed base plate and parallel moveable top plate with flat steel surface with sufficient friction to permit the development of the required shear forces;
 - (c) a means of measuring the mean thickness of the specimen to an accuracy of 0.01 mm.

As an alternative to (a) and (b), an appropriate inclined plane and kentledge system may be employed to produce the normal and shear loads.

The test procedure shall be as follows:

- (iv) Cut six representative specimens of minimum size 100 mm x 100 mm symmetrically about the core design. Three specimens shall be tested in accordance with (v) below and three in accordance with (vi) below.

- (v) The test specimen shall be placed symmetrically on the base plate and covered by the top plate. The means of measuring thickness shall be attached and the initial thickness measured.
- (vi) Apply the load smoothly and as quickly as possible to the top plate. The full load (normal and shear) shall be applied in less than 20 seconds and sustained for at least 1000 hours. The applied stresses shall be those given in Table 5/8. At least four measurements of thickness shall be made during each unit of logarithmic time after the first minute. Determine the long term thickness of the specimen as the thickness of the specimen at 1000 hours reduced by $2T$ where T is the difference in thicknesses of the specimen thickness recorded at 100 hours and 1000 hours. Repeat the test on the two other specimens. The test specimens shall be maintained at a constant temperature of $20^{\circ}\text{C} \pm 2^{\circ}\text{C}$ throughout the test period.
- (vii) Apply increasing increments of normal load to the specimen. Determine the short term equivalent load which shall be the load which when applied for a period of 20 ± 5 minutes produces a specimen thickness equal (within an accuracy of ± 0.05 mm) to the long term thickness of the specimen obtained at (v) above. Repeat the test on the other two specimens.
- (viii) The report shall include:
- (a) a reference to this method;
 - (b) sample identification details;
 - (c) the initial thickness of the sample;
 - (d) the applied load;
 - (e) the thickness of each sample at 100 and 1000 hours and the mean of the three results;
 - (f) a plot of percentage reduction in thickness against logarithmic time;
 - (g) the mean long term thickness;
 - (h) the mean short term equivalent load;
- (i) any deviations from the specified test procedure.
- Determining In-plane Flow Under Compressive Loading**
- 16** In-plane flow shall be determined in accordance with IS EN ISO 12958 except that the following conditions shall apply:
- (a) the applied normal stress shall be the greater of the value given in Table 5/8 (for sub-Clause 15) or the mean short term equivalent stress as determined in sub-Clause 14 (vi) of this Clause;
 - (b) the sample shall be tested such that the measured flow (or flows) is in the same direction as the principal flow (or flows) when the fin drain is in service;
 - (c) the foam rubber option of the test procedure shall be used (details of the foam rubber to be used may be obtained from the Employer's Representative);
 - (d) the hydraulic transmissivity shall be reported for each of the hydraulic gradients employed.
- Test Methods**
- 17** Notwithstanding the requirements of sub-Clauses 13, 14 and 15 of this Clause, variations in the test methods specified therein shall be made where deemed necessary by the Irish Agrément Board following consultation with the manufacturer. All such variations shall be recorded in the report.
- Certification**
- 18** Fin drains and constituent materials shall have a current Irish Agrément Board Certificate for Roads and Bridges or equivalent certifying the appropriate physical properties when tested in accordance with this Clause.

515 Narrow Filter Drains

General

- 1 Narrow filter drains shall comply with this Clause and the special requirements described in Appendix 5/4.

The term narrow filter drain refers to drain Types 8 or 9 indicated in NRA Road Construction Details listed in Appendix 5/7. They consist of a porous or perforated pipe laid in a narrow trench surrounded by granular material where the granular material and/or the pipe is enclosed by a layer of geotextile filter. Narrow filter drains and fin drains perform the same function.

Materials

- 2 The geotextile materials used in the drain shall be stored so that they are protected from the deleterious effects of short term exposure to ultraviolet light, and shall be resistant to degradation by acids, alkalis, common chemicals, bacteria, fungi and moulds occurring in soils and road construction materials.

After exposure to ultraviolet light the Employer's Representative may require evidence that the materials still comply with the requirements of this Clause. They shall be protected from damage and ultraviolet light and be labelled to identify the grade and manufacturer or supplier.

- 3 The geotextile used in narrow filter drains shall comply with all requirements of sub-Clause 514.4 for geotextiles used in fin drains.

For drain Type 8, in accordance with the NRA Road Construction Details listed in Appendix 5/7, the geotextile surround to the pipe shall consist of a prefabricated continuous close fitting sock. Alternatively the pipe shall be firmly wrapped in a single layer of geotextile with an overlap of between 50 and 75 mm and secured around the pipe in a manner so as to prevent the ingress of soil particles or other extraneous material and without affecting the permeability of the wrapped material. Splices between lengths of sock or layer shall have overlaps within these dimensions and be securely tied.

For drain Type 9, in accordance with the NRA Road Construction Details listed in Appendix

5/7, the geotextile surround to the granular material shall have a minimum overlap of 250 mm including 100 mm down-tuck. Splicing of lengths of geotextile shall consist of minimum 600 mm overlap secured with pins or mechanical ties. Where an outlet pipe passes through the geotextile a separate piece of geotextile shall be wrapped round the outlet pipe, flared against the geotextile in the filter drain and secured. Where drain lengths are terminated at chambers, the geotextile shall be secured against the chamber walls by suitable means so as to prevent the ingress of soil particles or other extraneous material into the drain.

- 4 Pipes and fittings shall comply with sub-Clauses 514.7 and 514.8. Where coilable pipes to BS 4962 : 1989 are used they shall be capable of being straightened so as to lie flat without restraint in the trench bottom before backfilling.
- 5 The granular material used for trench infill shall comply with the requirements for non-plasticity, LA category and sulphate content of sub-Clause 505.3 and have a grading within the limits of Table 5/9. The material when tested in accordance with sub-Clause 509.9 shall meet the permeability requirements described in Appendix 5/4.

TABLE 5/9: Narrow Filter Drain: Trench Infill Grading Requirements

| RCD Drain Type | Type 8 | Type 9 |
|--|---------------------------|--|
| Maximum Particle Size mm | 40 | 40 |
| D5 Size mm | >0.125 | >0.125 |
| D15 Size mm | As stated in Appendix 5/4 | |
| D85 Size mm | - | > Hole Diameter or 1.2 Times Width of Slot in Pipe |
| Uniformity Coefficient (C _u) D60/D10 | 5 to 12 | 5 to 12 or if C _u <5 D5>1mm |

D_n = Sieve size passing n% by weight of material

Dimensions

- 6 Unless otherwise described in Appendix 5/4 the dimensions of the narrow filter drain shall be as shown on the NRA Road Construction Details listed in Appendix 5/7. The pipe diameter shall be as stated in Appendix 5/4. The drain slope angle (α), as shown on the NRA Road Construction Details listed in Appendix 5/7, shall be not greater than 15% from the vertical unless otherwise stated in Appendix 5/4.

Installation and Handling

- 7 Narrow filter drains shall be installed as shown on the NRA Road Construction Details listed in Appendix 5/7. Before during and after installation the geotextile shall be protected from contamination, damage and exposure to ultraviolet radiation in accordance with sub-Clause 514.11.

The excavated trench bottom shall be free of irregularities and to the required levels given in Appendix 5/4. Rock and other hard protuberances shall be removed and any excess cut in the trench bottom filled and compacted back to the required grade with suitable excavated or imported material. Drain Type 9, in accordance with the NRA Road Construction Details listed in Appendix 5/7, shall have trench walls sufficiently clean to enable the geotextile to come into close contact with the wall when the granular material is placed inside it. The deposition and compaction of infill shall be in accordance with sub-Clause 505.4 for filter drains.

Narrow filter drains installed as part of the permanent Works shall not be used for the disposal of surface water run-off during construction. Narrow filter drains exposed to surface water ingress shall be temporarily protected.

Marker tapes shall conform and be installed in accordance with sub-Clause 514.11.

- 8 The Contractor shall obtain and make available the information required in sub-Clause 514.12 in respect of the geotextile and pipe type used in narrow filter drains.

Certification

- 9 The geotextile shall have a current Irish Agrément board Certificate certifying the

appropriate physical properties when tested in accordance with this Clause.

516 Combined Drainage and Kerb Systems

- 1 The Contractor shall design the combined drainage and kerb systems listed in Appendix 1/11 in accordance with IS EN 1433 and the design requirements given in Appendix 5/5.
- 2 Combined drainage and kerb systems shall permit lateral entry of surface water from the channel either continuously or at intervals not exceeding 1 m.
- 3 Combined drainage and kerb systems shall be suitable for their intended application and place of installation in the Works. The Contractor shall submit evidence of such suitability for purpose to the Employer's Representative.
- 4 Proprietary systems shall have a current Irish Agrément Board Roads and Bridges Certificate (or equivalent) and shall be laid and jointed in accordance with the manufacturer's written instructions.
- 5 Design flows given in Appendix 5/5 shall be accommodated without surcharge within the main combined drainage and kerb system and beneath the underside of any inlet slot sections. Combined drainage and kerb systems shall be designed to ensure a self cleaning velocity of 0.75m/s in the design flow.
- 6 Where applicable, the width and depth of the combined drainage and kerb system units shall not exceed the dimensions given in Appendix 5/5.
- 7 When used adjacent to porous asphalt surfacing materials, units shall incorporate side entry inlets to permit drainage of water held within the porous asphalt. Inlets shall comply with the requirements of Appendix 5/5 and shall have the capacity to drain porous asphalt.
- 8 Combined drainage and kerb systems, excluding in-situ system, shall be classified as follows according to their intended use and shall be as stated in Appendix 5/5:
- (i) C250; or
 - (ii) D400; or

- (iv) In exceptional circumstances where combined kerb and drainage units are to be located in areas subjected to large numbers of high speed heavy goods vehicles, Class E600 combined kerb and drainage units shall be considered.
- 9 Adjacent carriageway, footway, verge or central reserve construction shall not take place within 3 days of any bedding, backing, surrounding or jointing of combined drainage and kerb units.
- 10 Joints between units and between the channel and units, shall be designed to avoid leakage of surface water. Joints between bridge deck waterproofing and component parts passing through the waterproofing shall be watertight. Sealants shall be compatible with the waterproofing system.
- 11 Junctions, connecting pipes and other fittings comprising the combined drainage and kerb system shall comply with sub-Clause 501.3 and shall be as described in Appendix 5/5. The combined drainage and kerb system shall incorporate measures to enable rodding of the outfall pipework and adequate access for jetting by water jetting equipment of the system.
- 12 The system shall be cleaned out by high pressure water jetting or other appropriate means on completion of the Works. The system shall be left clean and free from all obstruction.

517 Linear Drainage Channel Systems

General

- 1 The linear drainage channel systems listed in Appendix 1/11 shall be treated as other features to be designed by the Contractor. The in-situ linear drainage channel systems shall conform, where applicable, to sub-Clauses 517.2 to 517.17 and the requirements in Appendix 5/6 and Appendix 1/11 as appropriate. The prefabricated linear drainage channel systems, where applicable, shall conform to IS EN 1433 and with the other requirements in Appendix 1/5.
- 2 Linear drainage channel systems shall be suitable for their intended use and place of installation in the Works. The Contractor shall submit evidence of such suitability to the Employers Representative in accordance with Appendix 1/5.
- 3 Design flows given in Appendix 5/6 shall be accommodated without surcharge within the main channel section and beneath the underside of any inlet slot sections.
- 4 Where applicable, the width and depth of linear drainage channel system units shall not exceed the dimensions given in Appendix 5/6.
- 5 Dimensions of inlet slots shall comply with the following criteria:
- (i) for prefabricated linear drainage channel systems they shall conform to IS EN 1433;
 - (ii) for in-situ linear drainage channel systems they shall conform to IS EN 1433.
- 6 When used adjacent to porous asphalt surfacing materials, units shall also incorporate side entry inlets to permit drainage of water held within the porous asphalt. Inlets shall comply with the requirements of Appendix 5/6 and shall have the capacity to drain porous asphalt.

Weathering Resistance

- 7 Linear drainage channel systems shall be marked in accordance with IS EN 1433 according to the grade of weathering resistance for drainage channels as stated in Appendix 5/6.

Classification

- 8 Linear drainage channel systems shall be classified as follows according to their intended use and shall be as stated In Appendix 5/6:
- (i) C 250; or
 - (ii) D 400; or
 - (iii) In exceptional circumstances where linear drainage channel systems have to be located in areas subjected to large numbers of high speed heavy goods vehicles, Class E 600 combined kerb and drainage units shall be considered.

Water Tightness

- 9 Joints between units comprising the system and between adjacent construction and the system shall be designed to avoid leakage of surface water. Where applicable, joints between bridge deck waterproofing and component parts passing through the waterproofing shall be watertight. Sealants shall also be compatible with the waterproofing system.
- 10 Junctions, connecting pipes and other fittings comprising the linear drainage system shall comply with the sub-Clause 501.3 and shall be as described in Appendix 5/6. The linear drainage channel system shall incorporate measures to enable rodding of the outfall pipework and adequate access for jetting by water jetting equipment into the system.

Cleaning

- 11 The system shall be cleaned out by high pressure water jetting or other appropriate means on completion of the Works. The system shall be left clean and free from all obstruction.

Manufactured Systems

- 12 Manufactured systems shall conform to IS EN 1433 and to the following:-
- (i) Products shall be designed and developed within an IS EN ISO 9001 system that has been assessed by a relevant recognised certification body. A relevant certification body shall have IS EN 45011 and IS EN ISO/IEC 17021 within its scope.
 - (ii) Products shall be type tested by a third party testing laboratory/organisation that has been accredited to IS EN ISO/IEC 17021 and has IS EN1433 within its scope.
 - (iii) The certification process shall be overseen or audited by a single relevant accredited certification body meeting the requirements in (i) and (ii) above.
 - (iv) Copies of all certificates shall be forwarded to the Employers Representative along with audited product test reports.

- (v) All documentation must be furnished in English or Irish.

In-Situ Systems

- 13 In-situ systems shall comply with the requirements of the 1100 Series except that the concrete shall be considered as plain concrete within the terms of this sub-Clause, irrespective of the inclusion of reinforcement.
- 14 In-situ systems shall be slip formed except for sections at gullies which shall be hand formed. Full depth joints shall be constructed at the interface at each side of gullies with a 25mm thick filler board complying with the requirements of the 1000 Series for Joint Filler Board. Joints shall be prepared and sealed in accordance with the 1000 Series.
- 15 The central void may be formed by an inflated tube which is then removed, or by an in-situ suitable pipe or similar former fit for the purpose. At gully positions the inner former shall be pre-sleeved with a suitable pipe of similar former fit for the purpose which shall be of sufficient length to overlap the two joins on either side of the gully.
- 16 The central void shall be checked in accordance with sub-Clause 509.4. Verification of slot widths shall be determined by suitable templates.
- 17 Reinforcement shall comply with the 1700 Series. Concrete cover shall be not less than 75mm.
- 18 Trial lengths and testing of in-situ channels shall be undertaken in accordance with clauses 6 and 8.6 of BS 5931 and the relevant paragraphs of clause 9 of IS EN 1433 respectively.

518 Thermoplastics Structured Wall Pipes and Fittings

General

- 1 Thermoplastics structured wall pipe shall comply with this Clause and the special

requirements described in Appendix 5/1. The term structured wall pipe shall mean all types of smooth bore pipe except solid wall homogeneous pipe. Typical forms of construction classified as structured wall pipes include: single wall externally structured smooth bore, twin wall, foamed core and spirally wound. The term fitting shall mean a product used in conjunction with the pipe to form the system but excluding gullies manhole chambers, inspection chambers and access chambers.

Materials

- 2 The materials from which the pipe and fittings are made, shall be treated so that they are protected from the deleterious effects of short term exposure to ultraviolet light, and shall be resistant to degradation by acids, alkalis, common chemicals, bacteria, fungi and moulds occurring in soil, road construction materials and road drainage systems. In addition, the materials from which the pipe and fittings are made, shall not incorporate any additives in quantities sufficient to cause microbiological degradation or to impair the conformity to the chemical, physical and mechanical properties or impact resistance requirements given in sub-Clause 5 of this Clause. The specification of the raw material shall be agreed between the certification body, as defined in sub-Clause 15 of this Clause, and the manufacturer and may incorporate re-processable and/or recyclable material. The agreed specification shall incorporate tolerances for each of the relevant characteristics defined in the appropriate clause of IS EN 13476. The Contractor shall submit, prior to commencement of the Works, completed information sheets in accordance with Appendix 5/8.

Dimensions

- 3 Systems for carrier drainage shall be between 150 mm and 900 mm nominal internal diameter. Pipes for narrow filter drains shall be 110 mm or 150 mm nominal internal diameter. Pipes for service ducts shall have nominal internal diameters of between 50 mm and 150 mm. Pipes for sub-soil drainage shall incorporate slots or holes with a minimum cross sectional area of 1000 mm² per metre run of pipe. The bore of the pipe and fittings shall be in accordance with the standard tolerances for nominal bores given in IS EN 476.

Appearance

- 4 The system shall have a smooth bore and be free from any burs, flash or other inconsistencies that could have a detrimental effect on the performance of the system. Pipes and fittings for drainage shall be externally coloured either terracotta or black. The colour of ducting pipes shall be in accordance with the UK National Joint Utilities Group publication "Guidelines on the Positioning and Colour Coding of Utilities' Apparatus". Any variation in the colour shall be described in Appendix 5/2.

Structured Wall Pipe

- 5 The structured wall pipe shall have the properties defined in Table 5/10.

TABLE 5/10: Requirements for Structured Wall Pipe

| Property | Relevant Standard | Requirement |
|---|---|--|
| General | IS EN ISO 2897-1, IS EN ISO 2897-2 and ISO 11922-1 | Dimensions to be specified |
| Ring stiffness | IS EN ISO 9969 | 6 kN/m ² minimum. |
| Creep Ratio | IS EN ISO 9967 | PVC-U - maximum 2.5 PP and PE - maximum 4.0 |
| High volume low pressure jetting | Sewer Jetting Code of Practice published by Water Research Centre Frankland Road Blagrove, Swindon Wilts. SN5 8YF UK. | Minimum acceptable failure pressure 137 bar. |
| Longitudinal bending | sub-Clause 520.11 | Pipes with nominal diameters ≤ 350 mm to have a difference in dimensions when measured in the vertical axis of less than 5% of the pipe length and no local permanent deformation occurs during the test. |
| Impact resistance at 0°C | IS EN 1411 with d25 striker of 1 kg | Preliminary test – test 10 pieces as described in IS EN 1411, dropping the striker from a height of 1m. If any test pieces fail, subject the pipe to the full test given in clause 7.3 of IS EN 1411 starting the striker from a drop height of 400 mm. The mean minus 1.64 times the standard deviation must exceed 1m. |
| Impact resistance at 23°C | IS EN 1411 with striker as above | Value to be derived from the same batch of pipe as used in the impact resistance test at 0°C. The H50 (mean) value - 10% to be used as the minimum value for quality control testing. Alternatively the 0°C test can be used as a QC test if the manufacturer chooses. |
| Rodding resistance | sub-Clause 520.12 | Pipes with nominal diameters ≤ 350 mm to have an average failure energy >3 joules |
| Static friction coefficient (ducts) | ESI (Electricity Supply Industry) 12-24 Test TT3 | Pass |
| Creep at elevated temperature | IS EN ISO 9967 | Test to be carried out at 45°C, creep ratio to be less than 2 times the (ducts) values to IS EN ISO 9967. |
| Resistance to point loads (ducts) | sub-Clause 520.13 | No perforation at 10% rod travel |
| Tensile strength of a seam | IS EN 1979 | IS EN 13476 |

Fittings

6 The fittings for use with structured wall pipe shall have the properties defined in Table 5/11.

TABLE 5/11: Requirements for Fittings

| Property | Relevant Standard | Requirement |
|---|--|--|
| General | IS EN ISO 2897-1, IS EN ISO 2897-2 and ISO 11922-1 | Dimensions to be specified |
| Ring stiffness (excluding couplers) | ISO/DIS 13967 | 6 kN/m ² minimum |
| Rodding resistance | sub-Clause 520.12 | Fittings with nominal diameters ≤ 350 mm to have an average failure energy >3 joules |
| Strength and flexibility of fabricated fittings | IS EN 12256 | IS EN 12256 |
| Impact resistance (drop test) | Drop Test to IS EN 12061: 1999 | Fall height 1000 mm at a temperature of 0°C. Product less than ND 300 shall show 'no damage'. Others may fail but must be identified as 'handle with care' |
| Watertightness of fabricated fittings | IS EN 1053 | 0.5 bar for 1 minute |

Pipe and Fittings

- 7 The pipe and fittings shall have the properties defined in Table 5/12

TABLE 5/12 Requirements of the Systems

| Property | Relevant Standard | Requirement |
|---|--|--|
| Leaktightness of joints – diameter distortion (watertight joints) | IS EN 1277 Method 4 Condition B Temperature (23 ± 2)°C | Use default values from ISEN 1277 |
| Leaktightness of joints – angular deflection (watertight joints) | IS EN 1277 Method 4 Condition C Temperature (23 ± 2)°C | Use default values from IS EN 1277 |
| Leakage rate from partially-watertight joints | sub-Clause 509.7 | Less than 20 times the square of the internal diameter of the pipe in metres shall flow through the joint in litres per minute |
| Resistance to wheel loads | IS EN 1437 adapted to suit NRA loading conditions | Less than 5% deformation when loaded to 100 kN (for unequal branches only) |

Bedding, Backfill and Surround Material

- 8 All systems shall be installed in accordance with the pipe and bedding combinations given in Advice Note HA 40 (UK DMRB 4.2.5). Other combinations shall be supported by calculations in accordance with IS EN 1295-1. Bedding, backfill and surround materials are classified in Clause 503 and Clause 505.

Installation and Handling

- 9 The bedding, surround and backfill shall be installed so as to cause no damage to the pipes and fittings. Installation of the pipe and fittings, particularly, procedures for preparation and execution of jointing operations, shall be in accordance with the manufacturer's instructions.

Identification

- 10 The Contractor shall obtain and make available records with the following information for each separate consignment of structured wall pipe or fittings delivered to Site:

- (i) system name, ring stiffness grade/number and size;
- (ii) name and address of the system manufacturer;
- (iv) consignment number and delivery date; and
- (v) a copy of the site delivery note.

Test Method for Longitudinal Bending

- 11
- (i) The test specimen shall be a six metre length of pipe or the maximum length available from the manufacturer (if less than six metres).
 - (ii) The apparatus shall include:
 - (a) two level support blocks at least 250 mm wide and of sufficient height to allow the pipe to sag over its length without touching the ground;
 - (b) a means of measuring the vertical distance between the pipe at the centre of the span and a fixed point of reference to an accuracy of ±0.5 mm.
 - (iv) The test procedure shall be as follows:
 - (a) condition the specimen for at least 1 hour at 23°C ± 2°C;
 - (b) set the supports at a distance apart equal to the length of pipe minus 500 mm;
 - (c) place the pipe symmetrically on the supports;
 - (d) measure the distance between the top of the supports and the fixed point of reference, in the vertical axis through the centre line of the pipe;
 - (e) after a period of two minutes measure the distance between the

- underside of the pipe at mid span and the fixed point of reference;
- (f) record the difference in readings as a percentage of pipe length.
- (e) embed the specimen into the sand using a vibrator with the inner surface exposed and facing upwards;
- (f) locate the specimen such that the marked point of impact is under the centre line of the tup;

Test Method for Rodding Resistance (Internal Puncture)

12 (i) The test specimens shall be:

- (a) twenty specimens cut from the structured wall pipe, each specimen to be 242 mm to 246 mm in length and a quarter section of the circumference or;
- (b) twenty specimens cut from a number of identical fittings.

(ii) The apparatus shall include:

- (a) a standard drop weight pipe testing apparatus capable of dropping a tup from a height of 1 m;
- (b) a 300g tup which can be varied in 30g multiples with a striker consisting a steel rod 18 mm in diameter with a 9 mm hemispherical end;
- (c) a 250 mm x 250 mm box containing dry Leighton Buzzard sand (Garside quarry) such that there is at least 100 mm of sand beneath the test specimen when bedded down.

(i) Procedure shall be as follows:

- (a) mark the intended point on the inside of the pipe on the intersection of the centre lines. With profiled pipes the point of impact shall be at the point nearest the intersection that is midway between the ribs or in the middle of a hollow corrugation;
- (b) alternatively mark the intended point of impact on the section of fitting;
- (c) condition the test specimens in air at a temperature of $4^{\circ}\text{C} \pm 2^{\circ}\text{C}$ for a period of at least one hour before the test;
- (d) locate the box under the drop tube of the impact apparatus;

- (e) embed the specimen into the sand using a vibrator with the inner surface exposed and facing upwards;

- (f) locate the specimen such that the marked point of impact is under the centre line of the tup;

- (g) drop the tup from a height of 1 m on to the inner surface of the specimen within 10 seconds of removing the specimen from the conditioning environment;

- (h) examine the specimen for damage. Damage is defined as a puncture or crack of the inner layer. Ductile bending or “whitening” is not considered to be damage;

- (i) if the specimen exhibits no damage the next test shall be conducted with a tup of 30 g greater mass. If the specimen does exhibit damage the next test shall be conducted using a tup of 30 g smaller mass;

- (j) after having completed the 20 strikes calculate the average of the energies where a pass (no damage) was recorded and the average of the energies where a failure (damage) was recorded, then calculate the average of the two averages.

(iv) The test report shall include:

- (a) identification of the samples;
- (b) the overall average;
- (c) whether or not the specimens were damaged.

Test Method for Resistance to Sharp Objects

13 (i) Test specimens shall be three samples of duct each 300 mm long.

(ii) The apparatus shall include:

- (a) a compression testing machine;
- (b) a 4.7 mm diameter steel rod with an hemispherical end;
- (c) and a 120° steel vee block at least 300 mm long.

- (iii) The test method shall be as follows:
- (a) condition the test specimens for at least 1 hour at $23^{\circ}\text{C} \pm 2^{\circ}\text{C}$;
 - (b) insert the steel rod in the jaws of the moveable platen of the compression testing machine so that the hemispherical end protrudes at least 15% of the nominal pipe diameter;
 - (c) position the specimen in the vee block and placed directly below the steel rod, for twin-walled pipe the specimen shall be positioned firstly so that the rod strikes on the corrugation or rib, and secondly in the valley (where possible);
 - (d) set the machine to lower at a rate of 5 mm/min;
 - (e) allow the rod to travel into the pipe a distance equal to 10% of the nominal internal diameter of the pipe (T10 mm) or until the pipe wall is perforated. When the outer skin of a twin wall pipe is perforated before 10% is attained, the rod travel up to the point of failure shall be recorded (Tf mm) and the rod allowed to travel to the inner wall. The rod travel shall be continued to a total of 10% (ie T10 - Tf) or until complete penetration occurs.
- (iv) For each test specimen: pipe size, reference, maximum load and rod travel shall be recorded.

- 14 Notwithstanding the requirements of sub-Clauses 11, 12 and 13 of this Clause, variations in the test methods specified therein shall be made where deemed necessary by the Irish Agrément Board following consultation with the manufacturer and in agreement with the National Roads Authority. All such variations shall be recorded in the report.

Certification

- 15 Pipes and fittings shall have a current Irish Agrément Board Roads and Bridges Certificate certifying the appropriate physical properties when tested in accordance with this Clause.

519 Geotextiles for Filter Drains Materials

- 1 The geotextile materials used in filter drains shall be stored so that they are protected from the deleterious effects of short term exposure to ultraviolet light, and shall be resistant to degradation by acids, alkalis, common chemicals, bacteria, fungi and moulds occurring in soils and road construction materials. After exposure to ultraviolet light the Employer's Representative may require evidence that the materials still comply with the requirements of this Clause. They shall be protected from damage and ultraviolet light and be labelled to identify the grade and manufacturer or supplier.
- 2 The geotextile for filter drains shall:
- (i) sustain a tensile load of not less than 5.0 kN/m at break and have a minimum failure strain of 10% when determined in accordance with IS EN ISO 10319;
 - (ii) have a minimum puncture resistance of 1200 N when determined in accordance with IS EN ISO 12236;
 - (iii) have a size distribution of pore openings such that the apparent opening size 090 when determined in accordance with IS EN ISO 12956, or other appropriate test, is less than 300 microns, unless otherwise specified in Appendix 5/4;
 - (iv) allow water to flow through it, in either direction, normal to its principal plane at a rate of not less than 10 l/m²/s, unless otherwise specified in Appendix 5/4, under a constant head of water of 100 mm and a maximum breakthrough head of 50 mm when determined in accordance with IS EN ISO 12958.
- 3 Splicing of lengths of geotextile shall consist of minimum 600 mm overlap secured with pins or mechanical ties. Where an outlet pipe passes through the geotextile a separate piece of geotextile shall be wrapped round the outlet pipe, flared against the geotextile in the filter drain and secured. Where drain lengths are terminated at chambers, the geotextile shall be secured against the chamber walls by suitable means so as to prevent the ingress of soil particles or other extraneous material into the drain.

Installation and Handling

- 4 Filter drains shall be installed as shown on the NRA Road Construction Detail listed in Appendix 5/7. Before, during and after installation the geotextile shall be protected from contamination, damage and exposure to ultraviolet radiation in accordance with sub-Clause 517.1. No geotextile shall be exposed to daylight (or any source of ultraviolet radiation) for a period exceeding a cumulative total of 50 hours. Any geotextile exposed to daylight (or any source of ultraviolet light) for a period exceeding a cumulative total of 50 hours shall be replaced unless it can be demonstrated that the geotextile still complies with the requirements of this Clause.
- 5 Trench walls shall be sufficiently clean to enable the geotextile to come into close contact with the wall when the granular material is placed inside it. The deposition and compaction of infill shall be in accordance with sub-Clause 505.4 for filter drains.

520 Attenuation

- 1 Where attenuation ponds are to be provided as part of the design they shall comply with the special requirements described in Appendix 5/9.