The Assessment of Concrete Road Structures Affected by Alkali Silica Reaction

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NRA DMRB and MCDRW References

For all documents that existed within the NRA DMRB or the NRA MCDRW prior to the launch of TII Publications, the NRA document reference used previously is listed above under ‘historical reference’. The TII Publication Number also shown above now supersedes this historical reference. All historical references within this document are deemed to be replaced by the TII Publication Number. For the equivalent TII Publication Number for all other historical references contained within this document, please refer to the TII Publications website.
The Assessment of Concrete Structures Affected by Alkali Silica Reaction

June 2014
Summary:

This Advice Note provides criteria for the assessment of the concrete structures which have been affected by Alkali Silica Reaction.
PART 8

NRA BA 52/14

ASSESSMENT OF CONCRETE STRUCTURES AFFECTED BY ALKALI SILICA REACTION

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1. **INTRODUCTION**

**General**

1.1 Engineers Ireland and The Irish Concrete Society established a joint working party to examine the effects of alkali-silica reaction (ASR) in concrete in Ireland. In 1989, a questionnaire was circulated throughout the country to find structures significantly affected by ASR but no damage was reported. The Report of the Joint Working Party was published in 1991. Following the publication of the European Standard EN 206-1 in 2001, which required actions to be taken to prevent deleterious ASR, the joint working party met to update the 1991 document. Again, no cases of deleterious ASR were reported (Ref 1).

1.2 Various methods have been investigated for assessing ASR affected structures (Ref 3). However, the use of NRA BD 44 formulae in conjunction with the in-situ concrete compressive strength has been found to be as reliable as any approach. It is generally conservative as the strength of many structures with ASR is less affected by ASR than is concrete compressive strength. There are, however, a few situations where greater strength reductions than implied by concrete compressive strength could occur. These are considered in Chapter 2 of this Advice Note.

**Scope**

1.3 This document gives guidance on the assessment of structures affected by ASR. It is to be read in conjunction with NRA BD 44 and NRA BA 44, and should be used in the assessment of structures where the presence of ASR is confirmed.

**Implementation**

1.4 This Advice Note should be used in all assessments of structures or structural elements.
2. **STRENGTH ASSESSMENT**

**General**

2.1 Structures suspected of suffering from ASR should have a "Special Inspection" in accordance with NRA BA 35 and the NRA Eirspan Bridge Management System (Ref 2) before they are assessed. The information required from this is essentially as for unaffected structures. However, special attention should be given to the possibility of delamination and any evidence of this should be reported. Delamination and excessive cracking particularly in areas of high bond or shear stress should also be noted. In such areas attempts should be made to see if there are cracks in line with the reinforcement.

2.2 If cracking, even of the characteristic map form, is observed it does not necessarily indicate that ASR is the primary cause. Even in concretes which are known to be susceptible to ASR, such cracking may be due to other causes. It should not be assumed that ASR is the cause until other explanations have been eliminated. Guidance on the correct diagnosis of ASR is given in reference 4.

2.3 The strength reductions due to mild amounts of ASR are not great and expansions of less than 0.7mm/m, based on core expansion tests at 20°C and 100% relative humidity, do not normally cause any significant loss of strength. Structures with these mild amounts of ASR, i.e. with an estimated 0.7mm/m or less of free expansion, do not therefore require special investigation.

**Concrete Properties**

2.4 It is normally necessary to rely on a combination of cores and judgement to obtain estimates of concrete strength. Reference 5 gives advice on cores in ASR affected structures. Due to the cracks induced by the ASR, cores taken from ASR affected structures are liable to give rather variable results. However, the strength of the structure appears to be better indicated by strengths obtained from relatively intact cores. Chana and Koroboki (Ref 6), found that cores under-estimated strength even when excessively cracked cores were rejected. Where it is not possible to take sufficient cores to give reliable strength estimates, reliance will have to be placed on judgement. If the degree of ASR can be quantified into an estimated free expansion (see Reference 4), the strength loss from this can be conservatively estimated from Reference 4.

2.5 Some aspects of structural behaviour, such as bond and shear, are more closely related to tensile than compressive strength. However, tensile tests on cores taken from ASR affected concrete give results which are too variable to be of much practical use. The classification of a bar should be based on the worst case of corrosion if more than one position is being considered.

**Delamination**

2.6 In structures which are severely affected by ASR, delamination of cover concrete can occur. This is extremely rare and such delamination is much more frequently caused by corrosion of reinforcement. Even if the delamination is caused by ASR, it is liable to lead to reinforcement corrosion and to require remedial action. Strength assessment is therefore only required to ensure the immediate safety of the structure.

2.7 If the cover concrete is delaminating over significant areas the structure should be assessed ignoring the cover concrete in those regions. The bond with bars which are in the plane of delamination should also be ignored. The bars should also be ignored for the purposes of calculating the ultimate concrete shear stress, \( v_c \).
Bond Strength

2.8  The presence of ASR is one of many factors which affect bond strength. Detailed recommendations on the prediction of bond strength are given by Chana and Korobakis (Ref 7, 8). In general the recommendations of NRA BD 44 are safe in all sections with links. Where sections have no links and have low cover the NRA BD 44 values may be unsafe and special investigation is required.

Future Deterioration

2.9  With the possible exception of delamination, any reduction of strength due to ASR is as a result of the deterioration of concrete properties and not the expansion itself. The strength reduction is not progressive and, for structures which are regularly inspected in accordance with the National Roads Authority procedures, it will normally be sufficient to assess them on the basis of present condition with no allowance for future deterioration.
3. **REFERENCES**


2. **NRA Design Manual for Roads and Bridges (NRA DMRB)**

   - NRA BD 44 The Assessment of Concrete Road Bridges and Structures
   - NRA BA 44 The Assessment of Concrete Road Bridges and Structures
   - NRA BA 35 Inspection and Repair of Concrete Road Structures
4.  ENQUIRIES

4.1  All technical enquiries or comments on this document or any of the documents listed as forming part of the NRA DMRB should be sent by e-mail to infoDMRB@nra.ie, addressed to the following:

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